

**010000 GENERAL**

- CONFORM TO THE REQUIREMENTS OF THE ONTARIO BUILDING CODE 2012, O. REG. 332/12, INCLUDING O. REG. 88/19, AND ANY APPLICABLE ACTS OF AUTHORITY HAVING JURISDICTION.
- READ STRUCTURAL DRAWINGS IN CONJUNCTION WITH THE SPECIFICATIONS AND ALL OTHER CONTRACT DOCUMENTS.
- BEFORE PROCEEDING WITH WORK, CHECK ALL THE DIMENSIONS SHOWN ON THE STRUCTURAL DRAWINGS WITH THE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS AND REPORT DISCREPANCIES TO THE CONSULTANT. DO NOT SCALE THE DRAWINGS.
- REFER TO THE ARCHITECTURAL AND OTHER DRAWINGS FOR LOCATIONS AND DIMENSIONING OF OPENINGS AND SLEEVES NOT SHOWN ON THE STRUCTURAL DRAWINGS. ASSUME TYPICAL DETAILS APPLY, HOWEVER, OBTAIN THE CONSULTANT'S PRIOR APPROVAL BEFORE INSTALLING OPENINGS, SLEEVES, ETC. WHICH ARE NOT SHOWN ON STRUCTURAL DRAWINGS.
- SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR LOCATIONS OF PITS, BASES, SUMPS, TRENCHES, DEPRESSIONS, GROOVES, CURBS, CHAMFERS AND SLOPES NOT SHOWN ON STRUCTURAL DRAWINGS. ADJUST UNDERSIDE ELEVATIONS OF FOOTINGS AS REQUIRED TO AVOID UNDERMINING THE FOOTINGS AND FOUNDATIONS.
- HORIZONTAL AND VERTICAL DESIGN LOADS ARE NOTED. THEY SHALL NOT BE EXCEEDED DURING CONSTRUCTION.
- TYPICAL STRUCTURAL DETAILS SHALL GOVERN THE WORK. IF DETAILS DIFFER ON THE DRAWINGS, THE MOST STRINGENT SHALL GOVERN.
- CONTRACTOR TO PROVIDE AND BE SOLELY RESPONSIBLE FOR ALL TEMPORARY WORKS.
- THE INFORMATION SHOWN ON STRUCTURAL DRAWINGS PLUS THE REQUIREMENTS OUTLINED IN SPECIFICATIONS REPRESENT THE BUILDING IN ITS FINISHED STATE. CONTRACTOR TO REVIEW THESE REQUIREMENTS AND DETERMINE ALL TEMPORARY WORKS REQUIRED TO COMPLETE THE STRUCTURE PER CONTRACT DOCUMENTS INCLUDING MEANS, METHODS, TECHNIQUES, SEQUENCES, PROCEDURES, TEMPORARY SHORING AND/OR BRACING, TEMPORARY OPENINGS, EXCAVATION SHORING, ERECTION PROCEDURES, ETC.
- SEE SPECIFICATIONS FOR DETAILED REQUIREMENTS.

**010001 DESIGN NOTES**

- THE BUILDING IS DESIGNATED AS BELONGING TO THE NORMAL IMPORTANCE CATEGORY, AS DEFINED IN THE OBC 2012.
- ALL REINFORCED CONCRETE ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH CSA STANDARD A23.3.
- ALL STRUCTURAL STEEL ELEMENTS HAVE BEEN DESIGNED IN ACCORDANCE WITH CAN/CSA-S16.
- LATERAL FORCES ON STRUCTURAL FRAME
  - THE LATERAL FORCES ARE RESISTED BY THE STEEL BRACED FRAME AND MOMENT FRAME SYSTEMS.
  - THE FRAME IS NOT STABLE UNTIL THE LATERAL LOAD RESISTING SYSTEM IS IN PLACE.
  - WIND:
    - THE DESIGN OF THE STRUCTURE FOR WIND IS BASED ON AN HOURLY WIND PRESSURE OF 0.32 kPa (BASED ON 1/50 YEAR RETURN).
    - EXPOSURE CONDITION: ROUGH TERRAIN.
    - THE IMPORTANCE FACTOR, *I<sub>w</sub>*, FOR WIND DESIGN IS 1.0. FOR DEFLECTION ANALYSIS, THE FACTOR IS 0.75.
    - THE DESIGN WIND FORCES HAVE BEEN CALCULATED IN ACCORDANCE WITH THE ONTARIO BUILDING CODE 2012 AND WITH THE STATIC PROCEDURE DESCRIBED IN THE USER'S GUIDE - NBC 2010 - STRUCTURAL COMMENTARIES (PART 4).
  - EARTHQUAKE:
    - THE DESIGN OF THE STRUCTURE FOR EARTHQUAKE IS BASED ON:
      - I<sub>e</sub>* = 1.0
      - SITE CLASS = D
      - S<sub>a</sub>*(0.2) = .151
      - S<sub>a</sub>*(0.5) = .105
      - S<sub>a</sub>*(1.0) = .063
      - S<sub>a</sub>*(2.0) = .032
      - PGA = .090
      - R<sub>d</sub>* = 1.5
      - R<sub>o</sub>* = 1.3
      - F<sub>a</sub>* = 1.24
      - F<sub>v</sub>* = 1.55
      - M<sub>v</sub>* = 1.0
    - THE SEISMIC HAZARD INDEX FOR THIS SITE IS:
      - I<sub>E</sub>F<sub>a</sub>S<sub>a</sub>*(0.2) = 0.24
    - THE STRUCTURE HAS BEEN DESIGNED FOR:
      - N/S DIRECTION
        - BASE SHEAR = 385 kN
        - BASE MOMENT = 2545 kNm
      - E/W DIRECTION
        - BASE SHEAR = 385 kN
        - BASE MOMENT = 2545 kNm
    - THE DESIGN EARTHQUAKE FORCES HAVE BEEN CALCULATED IN ACCORDANCE WITH THE ONTARIO BUILDING CODE 2012.
    - THE BUILDING'S STRUCTURAL CONFIGURATION IS DESIGNATED AS REGULAR.
- LATERAL FORCES ON FOUNDATION WALLS
  - WALLS RETAINING EARTH ARE DESIGNED TO SAFELY WITHSTAND A HORIZONTAL PRESSURE AT ANY DEPTH (h) GIVEN BY THE EXPRESSION:
 
$$P = K (\gamma h + q) \quad \text{WHERE}$$

K IS THE LATERAL EARTH PRESSURE COEFFICIENT  
P IS THE PRESSURE EXERTED HORIZONTALLY  
h IS THE DEPTH BELOW GRADE  
γ IS THE UNIT WEIGHT OF SOIL  
q IS THE SURCHARGE ON THE GROUND SURFACE
  - THE ADDITIONAL SEISMIC PRESSURE CONSIDERED IN CONJUNCTION WITH THE STATIC PRESSURE ABOVE IS GIVEN BY THE EXPRESSION:
 
$$P = 0.75 k \gamma (H - h) \quad \text{FOR A NON RIGID WALL}$$

$$P = 0.25 k \gamma H \{1 - [(H - 2h)/H]^2\} \quad \text{FOR A RIGID WALL}$$

WHERE

$$k = \frac{K}{\gamma H} \text{ IS THE DESIGN PEAK HORIZONTAL GROUND ACCELERATION COEFFICIENT (F<sub>a</sub> x PGA)}$$

H IS THE HEIGHT OF GRADE ABOVE THE LOWEST LATERAL RESTRAINT
- FOUNDATION AND OTHER WALLS RETAINING EARTH HAVE BEEN DESIGNED FOR SURCHARGE OF 12 kPa.
- THE WALLS HAVE BEEN DESIGNED ASSUMING THAT THERE IS FREE-DRAINING BACKFILL, OR THAT OTHER PROVISIONS HAVE BEEN MADE, SUCH THAT THE WALLS ARE NOT SUBJECT TO HYDROSTATIC PRESSURE.
- SNOW LOADS ON ROOFS
  - THE ROOFS HAVE BEEN DESIGNED WITH *S<sub>s</sub>* = 3.1 kPa AND *S<sub>r</sub>* = 0.4 kPa.
  - THE IMPORTANCE FACTOR, *I<sub>s</sub>*, IS 1.0 FOR ULS AND 0.9 FOR SLS.
  - ADDITIONAL SNOW ACCUMULATIONS ADJACENT TO HIGHER WALLS, ROOFS AND MECHANICAL UNITS ARE INDICATED ON THE DRAWINGS.
- RAINWATER LOADS ON ROOFS
  - THE ROOFS HAVE BEEN DESIGNED FOR NO FLOW.
- WIND UPLIFT OF ROOFS
  - ALL ROOF ELEMENTS INCLUDING JOISTS, METAL DECK, AND THEIR CONNECTIONS TO THE STRUCTURE ARE TO BE DESIGNED FOR UPWARD SUCTION DUE TO WIND. THE NET UPWARD DESIGN PRESSURES ARE SHOWN ON THE KEY PLAN BELOW.
- LIVE AND OTHER LOADS
  - SEE NOTES BELOW FLOOR PLANS.

**10. FUTURE EXTENSIONS**

- THE STRUCTURE HAS NOT BEEN DESIGNED FOR ANY FUTURE EXTENSIONS.

**010004 SUBMITTALS**

- GEOMETRY
  - SUBMIT SURVEY RECORDS CONFIRMING THAT THE BUILT GEOMETRY MATCHES THE DESIGN GEOMETRY.
- CONCRETE AND REINFORCEMENT
  - SUBMIT REINFORCING PLACING DRAWINGS AND BAR LISTS FOR REVIEW BY THE CONSULTANT.
  - PROVIDE TEST CYLINDERS IN ACCORDANCE WITH CAN3-A23.1 BUT A MINIMUM OF 3 CYLINDERS FROM EACH LOAD OF CONCRETE, TO BE TESTED; 1 AT 7 DAYS AND 2 AT 28 DAYS.
- STRUCTURAL STEEL
  - DESIGN DETAILS, CONNECTIONS, AND THE LIKE IN ACCORDANCE WITH THE ONTARIO BUILDING CODE FOR THE FORCES SHOWN ON THE DRAWINGS.
  - SUBMIT SKETCHES AND DESIGN CALCULATIONS STAMPED AND SIGNED BY QUALIFIED PROFESSIONAL ENGINEER LICENSED IN PROVINCE OF ONTARIO FOR NON STANDARD CONNECTIONS.
  - SUBMIT SHOP, ERECTION, AND SETTING DRAWINGS FOR REVIEW BY THE CONSULTANT.
  - ENSURE FABRICATOR DRAWINGS SHOWING DESIGNED ASSEMBLIES, COMPONENTS AND CONNECTIONS ARE STAMPED AND SIGNED BY QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
- STEEL JOISTS
  - DESIGN STEEL JOISTS, BRIDGING, AND THE LIKE IN ACCORDANCE WITH THE ONTARIO BUILDING CODE FOR THE FORCES SHOWN ON THE DRAWINGS.
  - SUBMIT SHOP DETAILS AND ERECTION DRAWINGS FOR REVIEW BY THE CONSULTANT.
  - SUBMIT DRAWINGS STAMPED AND SIGNED BY QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
- METAL DECK
  - DESIGN DECK IN CONFORMANCE WITH THE REQUIREMENTS OF CAN/CSA-S136, FOR THE FORCES SHOWN ON THE DRAWINGS.
  - SUBMIT SHOP DRAWINGS STAMPED AND SIGNED BY QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO.
- LIGHTWEIGHT STEEL FRAMING
  - SUBMIT SHOP AND ERECTION DRAWINGS BEARING THE SEAL OF A PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO, FOR REVIEW BY THE CONSULTANT.

**030000 CONCRETE**

- MATERIALS
    - CONCRETE
      - CONFORM TO THE REQUIREMENTS OF CSA STANDARD A23.1 (LATEST VERSION) AND THE FOLLOWING FOR STRENGTH, SLUMP, WATER-TO-CEMENTING MATERIALS CONTENT AND AIR CONTENT.
      - NOMINAL MAXIMUM SIZE OF AGGREGATE SHALL BE 20 mm. USE SMALLER AGGREGATES AS APPROPRIATE IN AREAS OF CONGESTED REINFORCING STEEL OR TO IMPROVE WORKABILITY. MODIFY MIX DESIGNS TO SUIT.
- | CATEGORY | DESCRIPTION               | EXPOSUR E CLASS PER A23.1 | CONCRETE STRENGTH f <sub>c</sub> (MPa) | MAX. W/C RATIO | AIR CONTENT <sup>1</sup> | SCOPE   |
|----------|---------------------------|---------------------------|--|----------------|--------------------------|---|
| CM 1     | FOUNDATION MIX            |                           | 25                                     |                | 5%-8%                    | FOOTINGS AND CAPS   |
| CM 2     | SLAB ON GRADE MIX         |                           | 25                                     |                |                          | INTERIOR SLABS ON GRADE   |
| CM 5     | TOPPING MIX               |                           | 20                                     |                |                          | TOPPINGS ON CONCRETE  |
| CM 8     | PARKING SLAB AND BEAM MIX | C-1 <sup>2</sup>          | 35                                     | 0.40           | 5%-8%                    | FOUNDATION WALLS ADJACENT TO PAVING; FRAMED SLABS AND BEAMS EXPOSED TO DE-ICING CHEMICALS.    |
| CM 9     | PAVING MIX                | C-2                       | 32                                     | 0.45           | 5%-8%                    | EXTERIOR PAVING AND SIDEWALKS   |
| CM 13    | EXTERIOR WALL MIX         | F-2                       | 25                                     | 0.55           | 4%-7%                    | FOUNDATION WALLS AND OTHER WALLS EXPOSED TO FREEZE THAW BUT NOT EXPOSED TO DE-ICING CHEMICALS |
| CM 14    | LEAN MIX                  |                           | 0.4 max. <sup>3</sup>                  |                | 4-6% (EXTERIOR ONLY)     | UNSHRINKABLE FILL   |
| CM 15    | SELF CONSOLIDATING MIX    |                           | 30                                     |                |                          | FOR USE WHERE CONVENTIONAL VIBRATION IS NOT VIABLE  |
- WHERE AGGREGATES SMALLER THAN 14 mm ARE USED, INCREASE AIR CONTENT BY 1%
  - REINFORCED CONCRETE EXPOSED TO DE-ICING CHEMICALS TO HAVE DCI CORROSION INHIBITOR @ 1%*I<sub>cc</sub>* max. DOSAGE OR APPROVED EQUIVALENT
  - MAX. 25kg CEMENT/*I<sub>cc</sub>* m.
- REINFORCEMENT:
    - CONFORM TO THE REQUIREMENTS OF CSA STANDARD G30 SERIES.
    - REINFORCING BARS SHALL HAVE A MINIMUM YIELD STRENGTH *f<sub>y</sub>* = 400 MPa, AND WELDED WIRE FABRIC SHALL HAVE A MINIMUM YIELD STRENGTH OF *f<sub>y</sub>* = 386 MPa. SUPPLY IN FLAT SHEETS.
    - WHERE WELDING OF REBAR IS INDICATED, WELDABLE GRADE REBAR SHALL BE USED.
  - EXECUTION
    - CONCRETE AND REINFORCEMENT
      - PROVIDE DOWELS TO WALLS AND COLUMNS SIMILAR IN NUMBER, SIZE, AND SPACING TO THE VERTICAL STEEL IN THE WALL OR COLUMN EXCEPT WHEN NOTED OTHERWISE.
      - CONSTRUCTION JOINTS:
        - HORIZONTAL CONSTRUCTION JOINTS SHALL NOT BE MADE IN BEAMS OR JOISTS, UNLESS SHOWN OR REVIEWED BY THE CONSULTANT.
        - VERTICAL CONSTRUCTION JOINTS MAY BE MADE ONLY AT MID-SPAN OF BEAMS, JOISTS, AND SLABS UNLESS OTHERWISE SHOWN OR DIRECTED AND THEIR LOCATION SHALL BE REVIEWED BY THE CONSULTANT.
        - PROVIDE 38x89 KEYS AT CONSTRUCTION JOINTS UNLESS NOTED OTHERWISE.
      - NO SLEEVES TO BE PLACED VERTICALLY OR HORIZONTALLY THROUGH BEAMS WITHOUT BEING REVIEWED BY THE CONSULTANT.
      - NO OPENINGS SHALL BE MADE IN FLAT SLAB COLUMN STRIPS UNLESS SHOWN OR REVIEWED BY THE CONSULTANT.
      - WELDING OF REBAR SHALL BE DONE IN ACCORDANCE WITH CSA W186.
    - CONCRETE COVER TO REINFORCEMENT:
      - CONFORM TO THE REQUIREMENTS OF CSA STANDARD A23.1 (LATEST VERSION) AND THE FOLLOWING FOR COVER TO REINFORCEMENT (mm):
        - NOT EXPOSED (N) AND FOR FIRE RATING:
- | LOCATION OR MEMBER  | FIRE RATING (HOURS) |     |    |    |    |
|---|---------------------|-----|----|----|----|
|   | UP TO 1             | 1.5 | 2  | 3  | 4  |
| BEAMS AND GIRDERS (PRINCIPAL REINFORCEMENT) 35M AND SMALLER | 40                  | 40  | 40 | 40 | 50 |
| 45M   | 45                  | 45  | 45 | 45 | 50 |
| 55M   | 55                  | 55  | 55 | 55 | 55 |
| SLABS – 25M AND SMALLER                                     | 25                  | 25  | 25 | 35 | 40 |

30M	30	30	30	35	40
35M	35	35	35	35	40
45M	45	45	45	45	45
55M	55	55	55	55	55
COLUMNS (VERTICAL BARS) – 35M AND SMALLER	40	40	50	50	63
45M	45	45	50	50	63
55M	55	55	55	55	63
WALLS – 25M AND SMALLER	25	40	50	50	63
30M	30	40	50	50	63
35M	35	40	50	50	63
45M	45	45	50	50	63
55M	55	55	55	55	63
STIRRUPS AND TIES			30		

- ADDITIONAL COVER REQUIREMENTS AS APPLICABLE:
    - CONCRETE CAST AGAINST AND PERMANENTLY EXPOSED TO EARTH:
      - 35M BARS AND SMALLER: 75mm
      - 45M BARS AND LARGER: 2x THE NOMINAL BAR DIAMETER
    - CONCRETE EXPOSED TO CHLORIDES (C-1, C-3) (DOES NOT INCLUDE CONCRETE PROTECTED BY A WATERPROOFING MEMBRANE):
      - 30M BARS AND SMALLER: 60mm
      - 35M BARS AND LARGER: 2x THE NOMINAL BAR DIAMETER
      - EXPOSED TO EARTH OR WEATHER (F-1, F-2)
        - 25M AND SMALLER: 40mm
        - 30M BARS AND LARGER: 1.5x THE NOMINAL BAR DIAMETER
  - PROTECTION
    - PROTECT CONCRETE EXPOSED TO DE-ICING SALTS IN ACCORDANCE WITH THE FOLLOWING TABLE. REFER TO THE SPECIFICATION FOR SPECIFIC REQUIREMENTS FOR PROTECTION.
- | CATEGORY | DESCRIPTION             | SCOPE   |
|----------|-------------------------|---|
| CP 0     | UNPROTECTED CONCRETE    | ALL CONCRETE NOT DESIGNATED AS PROTECTED BELOW.   |
| CP 1     | EPOXY COATED REBAR      | NONE  |
| CP 2     | STAINLESS STEEL REBAR   | NONE  |
| CP 3     | DCI CORROSION INHIBITOR | ALL CONCRETE EXPOSED TO WEATHER AT GRADE (NOT PROTECTED BY A MEMBRANE) INCLUDING CURBS AND WALLS. |
| CP 4     | CATHODIC PROTECTION     | NONE  |
- WATERSTOPS
    - PROVIDE WATERSTOPS AT ALL CONCRETE JOINTS MORE THAN 600 MM BELOW GRADE.

**050000 STRUCTURAL STEEL**

- MATERIALS
  - WIDE FLANGE SHAPES - CONFORM TO THE REQUIREMENTS OF ASTM A992/A992M, Fy=345MPa
  - HSS MEMBERS - CONFORM TO THE REQUIREMENTS OF G40.21 350W CLASS C
    - NOTE THAT ASTM A500 IS NOT AN ACCEPTABLE ALTERNATE FOR HSS MEMBERS WITHOUT REVIEW AND RESIZING (INCREASED SECTION SIZE OR WALL THICKNESS) BY THE CONSULTANT.
    - HSS PRODUCED TO ASTM A1085 IS AN ACCEPTABLE ALTERNATE TO CSA G40.21 350W CLASS C.
  - CHANNELS AND ANGLES - CONFORM TO THE REQUIREMENTS OF CSA G40.21 GRADE 350W
  - PIPE - ASTM A53/A53M
  - BOLTS, NUTS AND WASHERS - [ASTM F3125, GRADE A325]<sup>1</sup>
  - WELDS- CONFORM WITH CSA W59-03
  - HEADED STUD- CONFORM TO CSA W59 APPENDIX H, WITH TENSILE STRENGTH OF 450MPa AND YIELD STRENGTH OF 350MPa
  - ANCHOR RODS - CONFORM TO THE REQUIREMENTS OF CSA G40.21 GRADE 300W UNLESS NOTED OTHERWISE.
  - ALL OTHER - CONFORM TO THE REQUIREMENTS OF CSA G40.21 GRADE 300W
  - STEEL JOISTS - CONFORM TO CAN/CSA-S16-09
  - METAL DECK: - CONFORM TO THE REQUIREMENTS OF CAN/CSA-S136-07.
    - SHERWIN WILLIAMS B66W1 DTM ACRYLIC PRIMER/FINISH
    - PPG PITT-TECH 90-712 DTM PRIMER/FINISH
  - SHOP PRIMER: PHENOLIC ALKDYD PRIMER
    - DEVGUARD 4360 LOW VOC UNIVERSAL PRIMER
    - SHERWIN WILLIAMS B50 KEM BOND HS UNIVERSAL METAL PRIMER
    - PPG AMERCOAT 185H UNIVERSAL PHENOLIC PRIMER
  - REPAIR PRIMER FOR APPLICATION IN THE FIELD, WATER BASED ACRYLIC:
    - DEVFLEX 4020PF DIRECT TO METAL PRIMER
    - SHERWIN WILLIAMS PRO-CRYL B66-310 SERIES UNIVERSAL PRIMER
    - PPG PITT-TECH PLUS 90-912 SERIES DTM INDUSTRIAL PRIMER
  - PRIMER FOR STEEL TO RECEIVE INTUMESCENT FIREPROOFING. DETERMINED TO BE ACCEPTABLE BASED ON ADHESION AND COMPATIBILITY CHARACTERISTICS UNDER LABORATORY CONDITIONS IN ACCORDANCE WITH ASTM D3359-09a2, METHOD A AND / OR ASTM D4541-09a1, AND APPROVED BY MANUFACTURER OF INTUMESCENT FIREPROOFING TO BE APPLIED.
  - PRIMER FOR STEEL TO BE GALVANIZED AND RECEIVE A PAINT FINISH:
    - SHERWIN WILLIAMS B71Y1 DTM WASH PRIMER
    - CARBOLINE SANITILE120 HEAVY DUTY BONDING PRIMER
    - PPG PITT-TECH 90-712 SERIES DTM PRIMER
  - COLD GALVANIZING COATING FOR REPAIR OF GALVANIZED SURFACES:
    - ZRC ZERO-VOC GALVANIZING COMPOUND AS MANUFACTURED BY ZRC WORLDWIDE, MARSHFIELD, MA
    - AERVOE INDUSTRIES, INC. 'LOW VOC COLD GALVANIZING COATING 93% ZINC
  - SHEET RUBBER FOR THERMAL SEPARATION AT STEEL CONNECTIONS: AB-563 EPDM, HARDNESS:60±5 SHORE "A" DUROMETER, 3mm THICKNESS UNLESS OTHERWISE INDICATED, AS MANUFACTURED BY AMERICAN BILTRITE OR APPROVED EQUIVALENT. DISTRIBUTED BY ROBCO (MISSISSAUGA) 905-864-8555, GOODALL (OSHAWA) 905-728-1658, OR CHAMBERS AND COOKE (MARKHAM) 905-475-1331.
  - HEAVY DUTY BITUMINOUS COATING WHERE IN CONTACT WITH SOIL: WOHL COATINGS BB-110 OR APPROVED EQUAL.
- EXECUTION
  - PROVIDE A MINIMUM BEARING OF 200 mm FOR ALL STEEL BEAMS BEARING ON MASONRY AND A MINIMUM OF 100 mm ON STRUCTURAL STEEL, UNLESS NOTED OTHERWISE.
  - CENTRE BEARING PLATES UNDER BEAMS, OR AS NOTED.
  - BEARING PLATE DIMENSION GIVEN FIRST INDICATES SIDE PARALLEL TO BEAM WEB.
  - NO STRUCTURAL STEEL SHALL BE CUT WITHOUT THE PERMISSION OF THE CONSULTANT.
  - WHERE COLUMNS ARE STABILIZED BY WALLS PROVIDE COLUMN ANCHORS AT BUTTING WALLS. PROVIDE TEMPORARY BRACING UNTIL WALLS ARE BUILT TIGHT TO COLUMNS.
  - PROVIDE FRAMING AROUND ALL OPENINGS IN METAL DECK AS SPECIFIED. REFER TO TYPICAL DETAIL 0504 FOR DETAILS. SEE ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR OPENINGS NOT SHOWN ON THE STRUCTURAL DRAWINGS.
  - PROVIDE FULL HEIGHT WEB STIFFENERS AT ALL BEAMS BEARING ON COLUMNS AND ALL BEAMS SUPPORTING COLUMNS. WEB STIFFENERS SHALL BE OF THE SAME SIZE AND THICKNESS AS THE COLUMN FLANGES AND SHALL ALIGN WITH THE FLANGES OF THE SUPPORTING COLUMN.
  - CONNECT BEAMS FOR THE FACTORED REACTIONS INDICATED ON THE DRAWINGS. IF BEAM REACTIONS ARE NOT INDICATED, THE CONNECTIONS SHALL BE DESIGNED FOR ONE-HALF THE TOTAL UNIFORM LOAD CAPACITY OF THE SIMPLE SPAN BEAM FOR THE GIVEN SPAN PRESENTED IN THE CISC HANDBOOK OF STEEL CONSTRUCTION. BOLTED CONNECTIONS SHALL HAVE A MINIMUM OF TWO BOLTS.

- STEEL SUPPLIER TO DESIGN AND PROVIDE INTERCONNECTION BETWEEN BUILT UP MEMBERS AS NOTED. WHERE NOT NOTED, STEEL SUPPLIER IS TO INTERCONNECT AS REQUIRED TO ENSURE ADEQUATE CAPACITY FOR THE DESIGN FORCES SHOWN OR IMPLIED IN THE DRAWINGS.
- STEEL SUPPLIER TO DESIGN CONNECTIONS OF SINGLE ANGLE MEMBERS FOR THE FORCES SHOWN OR IMPLIED IN THE DRAWINGS, SUCH THAT CONNECTIONS ARE MADE TO THE SAME LEG EACH END BY WELDING OR WITH A MINIMUM OF TWO BOLTS.
- DESIGNATE STEEL AS ARCHITECTURALLY EXPOSED IN ACCORDANCE WITH THE FOLLOWING TABLE. \*REFER TO THE SPECIFICATION FOR SPECIFIC REQUIREMENTS FOR ARCHITECTURALLY EXPOSED STRUCTURAL STEEL (AESS). \*REFER TO THE CISC GUIDE FOR SPECIFYING ARCHITECTURALLY EXPOSED STRUCTURAL STEEL, AVAILABLE AT www.cisc-icca.ca. IN PARTICULAR, REFER TO TABLE 1 - AESS CATEGORY MATRIX AND ASSOCIATED NOTES.

CATEGORY	DESCRIPTION	SCOPE
SSS	STANDARD STRUCTURAL STEEL	ALL STRUCTURAL STEEL NOT DESIGNATED AS AESS BELOW.
AESS 1	BASIC ELEMENTS	EXPOSED COLUMNS
AESS 2	FEATURE ELEMENTS (VIEWED AT A DISTANCE > 6m)	[SCOPE]

- \*THE ARCHITECT SHALL REVIEW THE AESS STEEL IN PLACE AND DETERMINE ACCEPTABILITY BASED ON THE CATEGORY AND VISUAL SAMPLES (IF APPLICABLE). ADVISE THE CONSULTANT THE SCHEDULE OF THE AESS WORK.
- APPLY FIELD PRIMER TO WELDS, BOLTS AND AT LOCATIONS WHERE ORIGINAL PRIMER IS DAMAGED, EXCEPT FOR STEEL WHICH IS TO BE LEFT UNCOATED.
- PRIMERS AND PAINTS USED IN MULTI-COAT SYSTEMS WHERE A FINAL SHOP OR FIELD PAINT FINISH IS TO BE APPLIED SHALL BE SELECTED AND PRE-APPROVED BY THE ARCHITECT BASED ON SURFACE PREPARATION, EXPOSURE CONDITIONS, AND COMPATIBILITY WITH OTHER COATINGS.

**053100 STEEL DECKING**

- MATERIALS
  - STEEL DECKING PER PLAN AND CONFORMING TO CAN/CSA-S136 AND THE FOLLOWING:
    - CSSBI 10M FOR ROOF DECKING.
    - CSSBI 12M FOR FLOOR DECKING.
  - MINIMUM ZINC COATING OF Z275 FOR EXTERIOR DECKING AND DECKING EXPOSED TO VIEW WITHOUT PAINTED FINISH.
  - MINIMUM ZINC COATING OF Z275 FOR INTERIOR DECKING NOT EXPOSED TO VIEW AND INTERIOR DECKING WITH FIELD APPLIED PAINT SYSTEM.
  - LACEMENT OF EXISTING DECK IS REQUIRED.
- EXECUTION
  - DESIGN DECK IN ACCORDANCE WITH THE REQUIREMENTS OF THE ONTARIO BUILDING CODE.
  - DESIGN AND CONNECT METAL EDGE AND CLOSURE STRIPS, METAL SCREEDS, FLASHINGS AND THE LIKE.
  - DESIGN FRAMING FOR 450mm OR SMALLER OPENINGS IN ROOF DECK, AND 300mm OR SMALLER OPENINGS IN FLOOR DECK. REINFORCE OPENINGS OVER 150mm, AS REQUIRED.
  - PLACE SHEETS IN MINIMUM 3 SPAN LENGTHS. BEAR ENDS MINIMUM 50mm.
  - LAP ENDS OF NON-COMPOSITE DECK UNITS A MINIMUM OF 50mm AND ONLY OVER SUPPORTING MEMBERS.
  - AS A MINIMUM, WELD DECK TO SUPPORTS AND PERIMETER ELEMENTS WITH 20mm PUDDLE WELDS AT MAXIMUM 400mm o/c OR EVERY SECOND FLUTE, WHICHEVER IS LESS.
  - AS A MINIMUM, FASTEN SIDE JOINTS OF DECK UNITS BETWEEN SUPPORTS BY CLINCHING AT 600mm INTERVALS OR WITH 25mm LONG WELDS AT 1000mm INTERVALS.
  - PAINT WELDS AND REPAIR DAMAGED COATING WITH GALVA-COAT COATING.
  - DO THE FOLLOWING WHERE DECKING IS EXPOSED TO VIEW:
    - LAP ENDS OF DECK UNITS ONLY OVER SUPPORTING MEMBERS. NO SEAMS ARE PERMITTED WITHIN SPANS.
    - KEEP DECK FREE OF DIRT, SCALE, FOREIGN MATTER, DENTS OR DEFORMATIONS.
    - KEEP FUSION WELDS WELL WITHIN THE BEARING WIDTH OF SUPPORTING MEMBERS.
    - AVOID WELD DAMAGE TO THE DECK OR ITS SUPPORTS.
- FOUNDATIONS
  - A SOIL INVESTIGATION HAS BEEN DONE BY REDSTONE ENGINEERING AS REPORTED IN THEIR SOIL REPORT NO. 21R110, DATED JUNE 18, 2021. READ THIS REPORT, AND BE THOROUGHLY FAMILIARIZED WITH ITS FINDINGS.
  - FOUND ALL FOOTINGS ON NATURALLY CONSOLIDATED UNDISTURBED SOIL OR ENGINEERED FILL CAPABLE OF SAFELY SUSTAINING AN ULTIMATE BEARING VALUE OF 135 kPa AND AN ALLOWABLE BEARING VALUE OF 90 kPa.
  - FOUND FOOTINGS EXPOSED TO FREEZING BELOW THE LEVEL AT WHICH POTENTIAL DAMAGE RESULTING FROM FROST ACTION CAN OCCUR, BUT A MINIMUM OF 1500 mm BELOW FINISHED GRADE IF NOT NOTED TO BE FOUNDED LOWER.
  - THE LINE OF SLOPE BETWEEN ADJACENT FOOTINGS OR EXCAVATIONS OR ALONG STEPPED FOOTINGS SHALL NOT EXCEED A RISE OF 7 IN A RUN OF 10. AT STEPS CONSTRUCT LOWER FOOTINGS PRIOR TO CONSTRUCTING HIGHER FOOTINGS.
  - PLACE SLABS ON GRADE ON MATERIAL CAPABLE OF SAFELY SUSTAINING 25kPa WITHOUT SETTLEMENT RELATIVE TO THE BUILDING FOUNDATIONS.
  - REFER TO GEOTECHNICAL REPORT FOR SUBGRADE REQUIREMENTS DIRECTLY BELOW SLAB ON GRADE.
  - DO NOT PLACE BACKFILL AGAINST WALLS RETAINING EARTH (OTHER THAN CANTILEVER WALLS) UNTIL THE FLOOR CONSTRUCTION AT TOP AND BOTTOM OF THE WALLS IS POURED AND HAS ATTAINED 70% OF ITS SPECIFIED STRENGTH.
  - CARRY OUT BACKFILLING AGAINST FOUNDATION WALLS WHERE THERE IS GRADE ON BOTH SIDES IN SUCH A MANNER THAT THE LEVEL OF BACKFILLING ON ONE SIDE OF THE WALL IS NEVER MORE THAN 500 mm DIFFERENT FROM THE LEVEL ON THE OTHER SIDE OF THE WALL.
  - PROVIDE FOOTINGS AS PER TYPICAL DETAIL 0306 FOR ALL LOAD BEARING MASONRY WALLS AND ALL NON-LOAD BEARING MASONRY WALLS THICKER THAN 190 mm. ALL NON-LOAD BEARING MASONRY WALLS 190 mm OR LESS SHALL REST ON A THICKENING OF THE SLAB ON GRADE AS PER THE TYPICAL DETAIL OR AS NOTED ON DRAWINGS.

Contractor must check and verify all dimensions on the job, and report any discrepancies to the Architect before proceeding with the work.		
Do not scale this drawing.		
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9	2022/05/30	ISSUED FOR STEEL RESTRUCTURING
8	2022/01/31	REVISED ISSUED FOR TENDER/ISSUED FOR BUILDING PERMIT
7	2021/09/28	ADDENDUM S2
6	2021/09/14	ADDENDUM S1
5	2021/09/09	ISSUED FOR BUILDING PERMIT
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3	2021/08/25	ISSUED FOR TENDER REVIEW
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TEL 416 487 8151

PROJECT NAME:  
**NEW SAYERS FOOD STORE BURLEIGH STREET, APSLEY**

PROJECT ADDRESS:  
**132 Burleigh Street**

SEAL:

DRAWN: DM	CHECKED: IFM
SCALE:	PROJECT NUMBER: 210112

SHEET TITLE:  
**GENERAL NOTES**

# S001

Contractor must check and verify all dimensions on the job, and report any discrepancies to the Architect before proceeding with the work.

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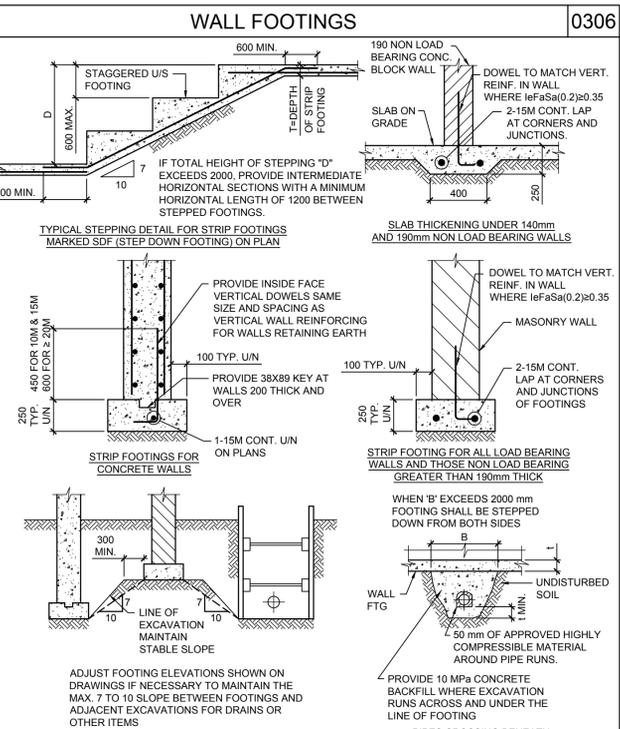
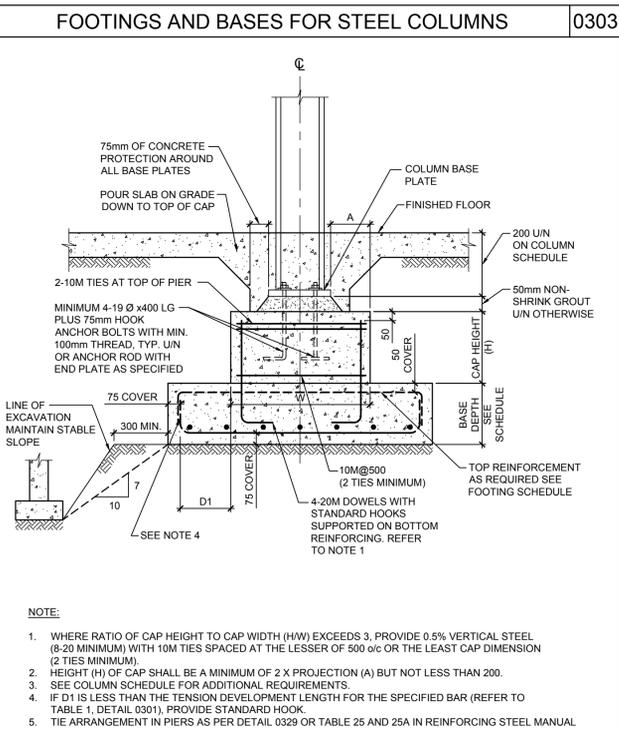
DRAWN: DM  
SCALE: PROJECT NUMBER: 210112

SHEET TITLE:  
**TYPICAL DETAILS**

**S002**

ABBREVIATIONS		0001	
A BOLT = ANCHOR BOLT	GA = GAUGE	R = RADIUS, REACTION	
ADJ = ADJUSTABLE	GALV = GALVANIZED	RAD = RADIUS	
ALT = ALTERNATE	GEN = GENERAL	REF = REFERENCE	
ARCH = ARCHITECTURAL	H. HOR = HORIZONTAL	REIN = REINFORCE	
ASL = ADDITIONAL SNOW LOAD	HEF, H EF = HORIZONTAL EACH FACE	REINFC = REINFORCEMENT	
@ = AT	HEE = HOOKED EACH END	REQ'D = REQUIRED	
B = BOTTOM	HF = FACTORED HORIZONTAL	REV = REVISION, REVISED	
BEW = BOTTOM EACH WAY	HSC = FORCE, KN	R/W, R/W = REINFORCED WITH	
BLL = BOTTOM LOWER LAYER	INT = INTERIOR	SECT = SECTION	
BUL = BOTTOM UPPER LAYER	ISO = HORIZONTALLY SLOTTED CONNECTION	SDF = STEP DOWN FOOTING	
BLDG = BUILDING	IF = INSIDE FACE	SL = SLAB	
BM = BEAM	INT = INTERIOR	SOG = SLAB ON GRADE	
BPL = BASE OR BEARING PLATE	JT = JOINT	SPECS = SPECIFICATIONS	
BSMT = BASEMENT	KN = KILONEWTON	STD = STANDARD	
c/c, o/c = CENTRE TO CENTRE	kg = KILOGRAM	STRUC = STRUCTURAL	
C/W = COMPLETE WITH	kN.m = KILONEWTON METRES	T = TOP	
C = EPOXY COATED	kN/m = KILONEWTON PER METRE	TEW = TOP EACH WAY	
CF = FACTORED COMPRESSIVE FORCE, KN	kPa = KILOPASCAL	TF = FACTORED TORSIONAL MOMENT, kNm	
CANT = CANTILEVER	LL = LIVE LOAD	TI = FACTORED TENSION FORCE, KN	
CA = COLUMN ABOVE	LG = LONG	TJ = TIE JOIST	
CB = COLUMN BELOW	LLV = LONG LEG VERTICAL	TLL = TOP LOWER LAYER	
COL = COLUMN	LLH = LONG LEG HORIZONTAL	TUL = TOP UPPER LAYER	
CONC = CONCRETE	MAX = MAXIMUM	TEMP = TEMPERATURE	
CONSTR = CONSTRUCTION	MC, M = MOMENT CONNECTION	TYP = TYPICAL	
CJ = CONSTRUCTION JOINT	MECH = MECHANICAL	UL = UPPER LAYER	
CONT = CONTINUOUS	MEZZ = MEZZANINE	UN = UNLESS OTHERWISE NOTED	
DET = DETAIL	MIN = MINIMUM	U/S = UNDERSIDE	
DIAG = DIAGONAL	MISC = MISCELLANEOUS	VI = FACTORED SHEAR FORCE, KN OR FACTORED VERTICAL BRACING FORCE, KN	
DIA, Ø = DIAMETER, BAR	ML = MIDDLE LAYER	V. VERT = VERTICAL	
DIAM, Ø = DIAMETER, BAR	MM = MILLIMETRE	VEF, V EF = FACTORED EACH FACE	
DIM = DIMENSION	MOM = MOMENT	VBR = VERTICALLY SLOTTED CONNECTION	
DJ = DOUBLE JOIST	MTR = METRE, METRIC	WPL = WALL PLATE	
DO = DITTO	MPa = MEGAPASCAL	WWF = WELDED WIRE FABRIC	
DL = DEAD LOAD	Mtx = FACTORED BENDING MOMENT ABOUT X-AXIS	WWM = WELDED WIRE MESH	
DWG(S) = DRAWING(S)	Mfy = FACTORED BENDING MOMENT ABOUT Y-AXIS	W/W, w = WITH	
DWL(S) = DOWEL(S)	N = NEWTONS	wd, wl = UNIFORMLY DISTRIBUTED LOADS	
EA = EACH	NIC = NOT IN CONTRACT		
EF = EACH FACE	NF = NEAR FACE		
EW = EACH WAY	N-S = NORTH-SOUTH		
EL = ELEVATION	NTS = NOT TO SCALE		
ELECT = ELECTRICAL	OP = OUTSIDE FACE		
ELEV = ELEVATOR	OWS = OPEN WEB STEEL JOISTS		
E-W = EAST-WEST	OPEN = OPENING		
EQ = EQUAL	PL = PLATE		
EXIST = EXISTING	PC = PRECAST		
EXP JT = EXPANSION JOINT	PROJ = PROJECTION		
EXT = EXTERIOR			
FF = FAR FACE			
FDN = FOUNDATION			
FL = FINISHED			
FLOOR = FLOOR			
FTG = FOOTING			

REINFORCEMENT DEVELOPMENT LENGTHS						0301	
<b>TABLE 1 - TENSION DEVELOPMENT (CLASS A) LENGTH (mm)</b>						<b>TABLE 2 - TENSION LAP SPICE (CLASS B) LENGTH (mm)</b>	
BAR SIZE	20MPa	25MPa	30MPa	35MPa	40MPa	45MPa	fc
10	320	300	300	300	300	300	
15	480	430	390	370	340	320	
20	640	580	530	490	460	430	
25	1010	900	820	760	710	670	
30	1210	1080	990	910	850	800	
35	1410	1260	1150	1060	1000	940	
45	1820	1620	1480	1370	1290	1210	
55	2220	1980	1810	1680	1570	1480	
TOP BAR VALUES ARE 1.3x THE ABOVE LENGTH INDICATED FOR ALL SLABS GREATER THAN 325mm IN THICKNESS						TOP BAR VALUES ARE 1.3x THE ABOVE LENGTH INDICATED FOR ALL SLABS GREATER THAN 325mm IN THICKNESS	
<b>TABLE 3 - COMPRESSION DEVELOPMENT LENGTH (mm)</b>						<b>TABLE 4 - COMPRESSION LAP SPICE LENGTH (mm)</b>	
BAR SIZE	20MPa	25MPa	30MPa	35MPa	40MPa	45MPa	fc
10	210	200	200	200	200	200	
15	320	290	260	260	260	260	
20	430	380	350	350	350	350	
25	540	480	440	440	440	440	
30	640	580	530	530	530	530	
35	750	670	620	620	620	620	
45	970	860	790	790	790	790	
55	1180	1060	970	970	970	970	
FOR 35M OR SMALLER BARS WHERE THE SIDE COVER IS NOT LESS THAN 60mm AND FOR 30M BARS WHERE THE COVER ON THE BAR EXTENSION BEYOND THE HOOK IS NOT LESS THAN 50 mm, LENGTHS MAY BE REDUCED BY A FACTOR OF x0.7.						REFER TO REINFORCING STEEL MANUAL OF STANDARD PRACTICE FOR MORE INFORMATION.	
<b>TABLE 5 - DEVELOPMENT LENGTH (mm) FOR STANDARD HOOKS</b>						<b>TABLE 6 - STANDARD HOOK DIMENSION FOR BLACK REINFORCING</b>	
BAR SIZE	20MPa	25MPa	30MPa	35MPa	40MPa	45MPa	400R OR 500R
10	220	200	180	170	160	150	
15	340	300	270	250	240	220	
20	450	400	370	340	320	300	
25	560	500	460	420	400	370	
30	670	600	550	510	470	450	
35	780	700	640	590	550	520	
45	1010	900	820	760	710	670	
55	1230	1100	1000	930	870	820	
BAR SIZE	90° HOOK (mm)	180° HOOK (mm)	90° HOOK (mm)	180° HOOK (mm)			400W OR 500W
10	180	140	180	130			
15	260	180	250	170			
20	310	220	300	200			
25	400	280	400	280			
30	510	400	480	350			
35	610	480	590	430			
45	790	680	770	630			
55	1030	900	1010	850			



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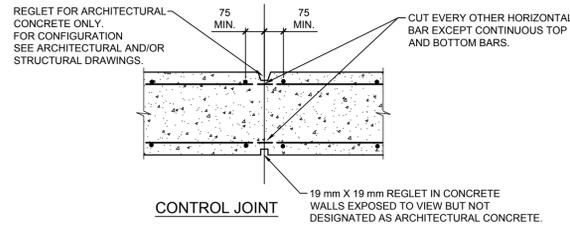
SEAL:

DRAWN: DM  
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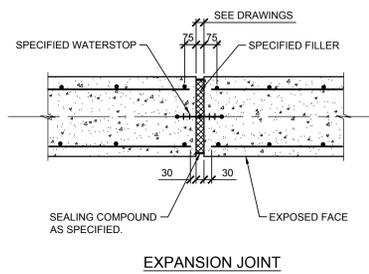
SHEET TITLE:  
**TYPICAL DETAILS**

**S003**

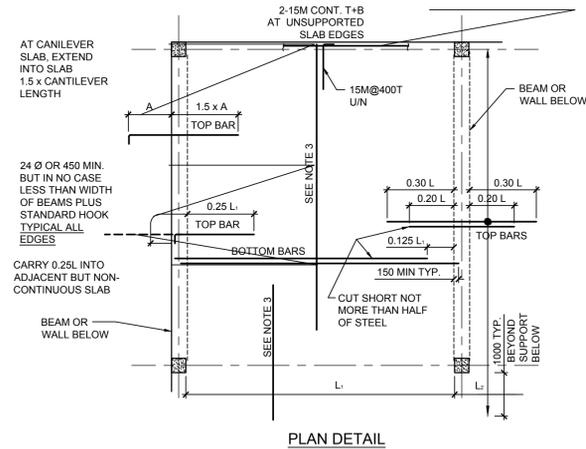
**CONTROL AND EXPANSION JOINTS IN CONCRETE WALLS** 0315



- NOTES:**
- AS A MINIMUM PROVIDE CONTROL JOINTS IN ALL FOUNDATION WALLS AT 6m MAX.
  - PROVIDE CONTROL JOINT AT ALL LOCATIONS MARKED THUS ▲ IN PLAN.
  - DO NOT PLACE CONTROL JOINTS IN SHAFT WALLS (WALLS THAT ARE NOT BRACED BY FLOOR SLABS).
  - DO NOT PLACE CONTROL JOINTS IN BEAMS AND COLUMNS.
  - CONFIRM WITH CONSULTANT LOCATIONS WHERE BARS TO BE CUT ARE LARGER THAN THE REST OF REINFORCING.



**ONE-WAY SLABS** 0317



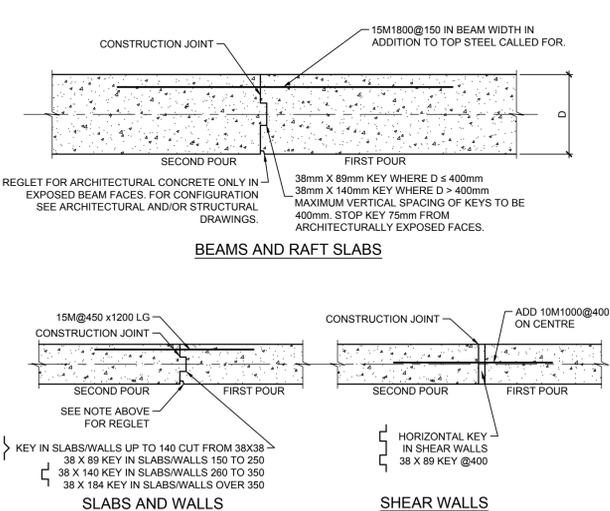
**REINFORCING FOR CRACK CONTROL**

SLAB THICKNESS	INTERIOR SLABS As = .002 Ag	SLABS EXPOSED TO WEATHER As = .0025 Ag
75	10 @ 500	10 @ 500
100	10 @ 500	10 @ 400
125	10 @ 400	10 @ 300
150	10 @ 325	15 @ 500
175	15 @ 500	15 @ 450
200	15 @ 500	15 @ 400
225	15 @ 500	15 @ 350
250	15 @ 400	20 @ 500
275	15 @ 350	20 @ 400
300	20 @ 500	20 @ 400

**NOTES:**

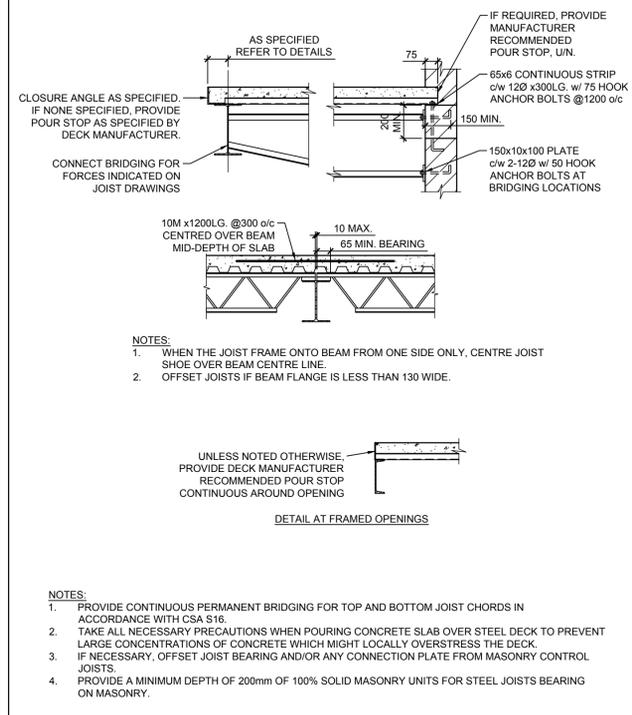
- TOP BARS SHALL BE CARRIED 24 Ø INTO THE SUPPORT OR HOOKED AS SHOWN DOTTED.
- SUPPORT ALL BARS IN ACCORDANCE WITH THE RSIC MANUAL OF STANDARD PRACTICE USING REBAR OR PLASTIC CHAIRS AND BOLSTERS ONLY.
- L IS GREATER OF L AND L.
- IN ALL ONE WAY SLABS PROVIDE TEMPERATURE REINFORCEMENT PERPENDICULAR TO THE SPAN AS PER THE TABLE BELOW, UNLESS OTHERWISE NOTED ON PLAN. LAP TEMPERATURE REINFORCEMENT 24 Ø, BUT NOT LESS THAN 300 mm.

**CONCRETE CONSTRUCTION JOINTS** 0325

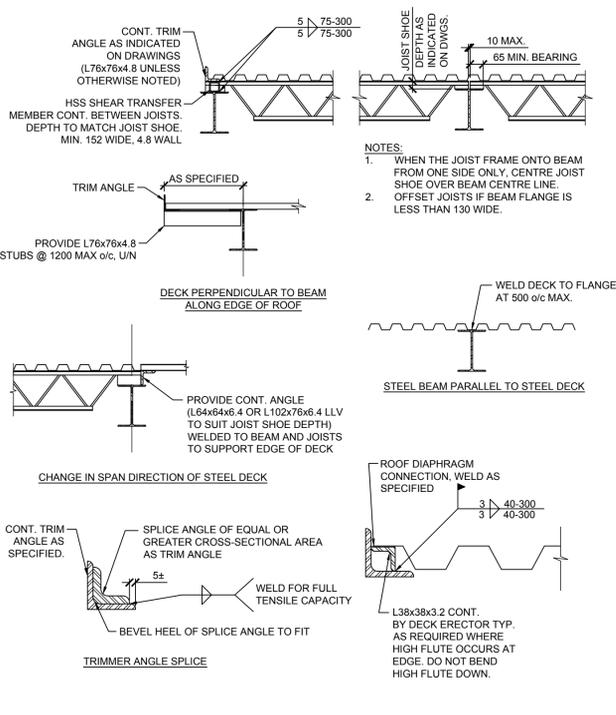


- NOTES:**
- SUBMIT CONSTRUCTION JOINT SHOP DRAWINGS FOR REVIEW BY CONSULTANT.
  - EXTEND NORMAL TOP AND BOTTOM REINFORCEMENT THROUGH JOINT FOR FULL LENGTH AS SHOWN ON DRAWINGS, SCHEDULES AND TYPICAL DETAILS.
  - CONSTRUCTION JOINTS TO BE LOCATED AT MIDDLE THIRD OF SPAN.
  - CONSTRUCTION JOINTS IN PAD FOOTINGS, IN SLABS UNDER AND PARALLEL TO WALLS, IN BEAMS AT SUPPORTS OR AT COLUMNS ABOVE ARE NOT PERMITTED, UNLESS APPROVED BY CONSULTANT.

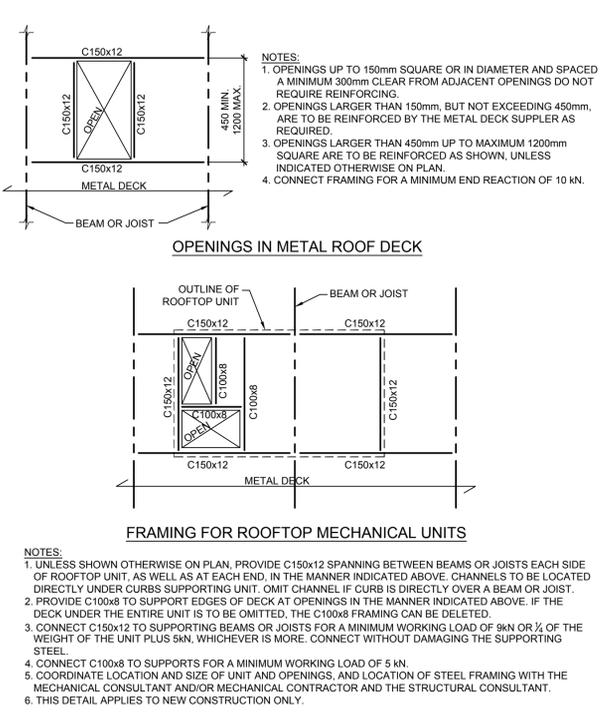
**CONCRETE SLAB ON STEEL DECK FRAMING DETAILS** 0501



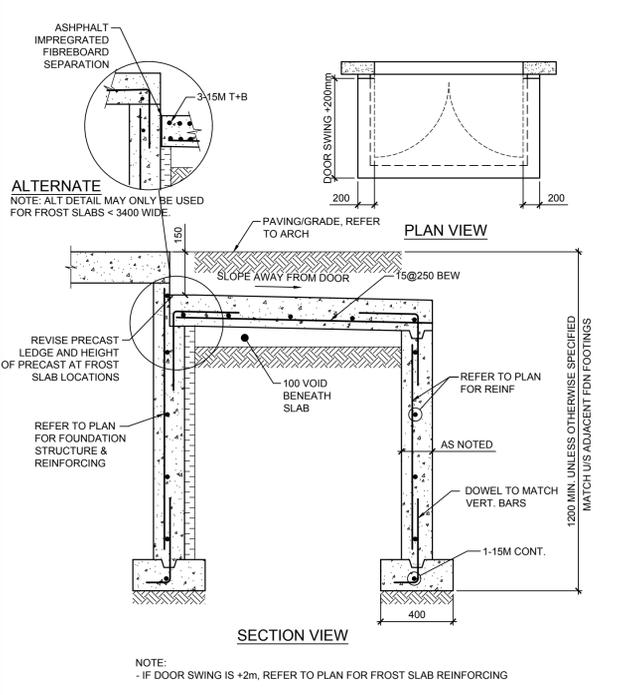
**STEEL DECK FRAMING DETAILS** 0503



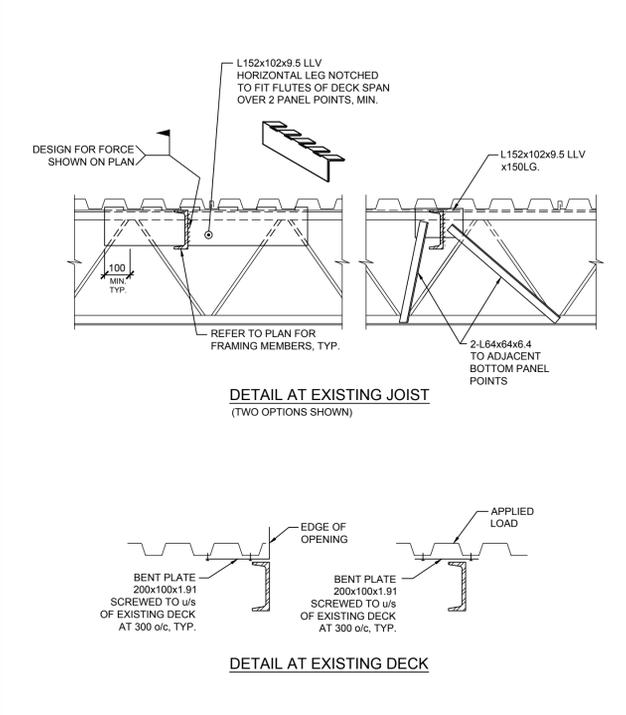
**FRAMING FOR ROOFTOP UNITS AND OPENINGS IN DECK** 0504



**FROST SLABS** 0355A



**FRAMING ONTO EXISTING STEEL JOISTS AND DECK** 0505



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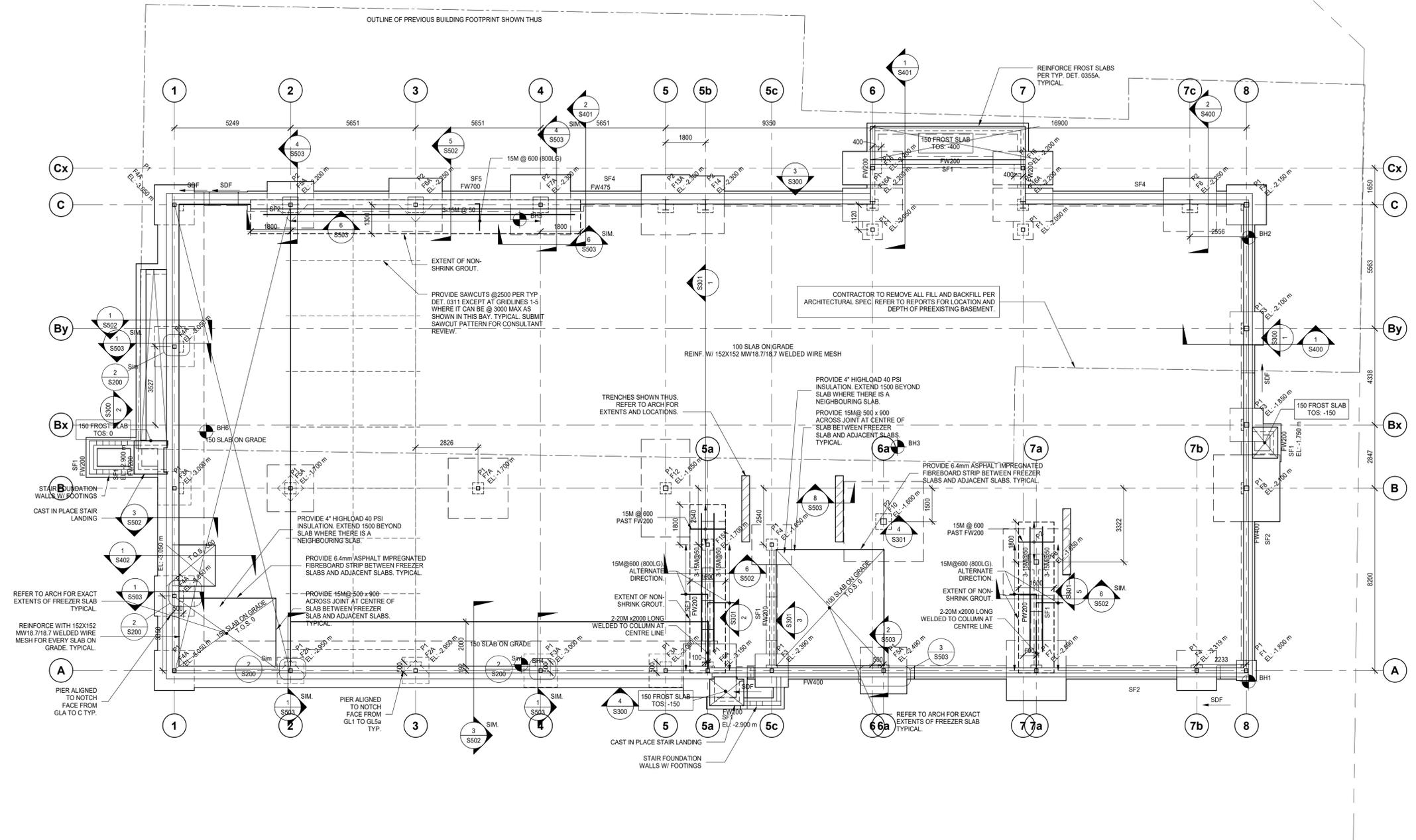
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SCALE:	PROJECT NUMBER: 210112

SHEET TITLE:  
**TYPICAL DETAILS**

# S004

<h3>CANTILEVERED BEAM CONNECTION</h3>	<h3>ANCHOR ROD WITH END PLATE</h3> <table border="1" style="margin: 10px auto;"> <thead> <tr> <th rowspan="2">ROD <math>\varnothing</math></th> <th colspan="3">PLATE DIMENSIONS</th> <th rowspan="2">WELD</th> </tr> <tr> <th>A</th> <th>B</th> <th>C</th> </tr> </thead> <tbody> <tr> <td>19 (3/4")</td> <td>60</td> <td>13</td> <td>10</td> <td></td> </tr> <tr> <td>25 (1")</td> <td>75</td> <td>16</td> <td>13</td> <td></td> </tr> <tr> <td>32 (1 1/4")</td> <td>95</td> <td>19</td> <td>17</td> <td></td> </tr> <tr> <td>38 (1 1/2")</td> <td>115</td> <td>22</td> <td>21</td> <td></td> </tr> <tr> <td>45 (1 3/4")</td> <td>135</td> <td>25</td> <td>24</td> <td></td> </tr> <tr> <td>51 (2")</td> <td>150</td> <td>29</td> <td>27</td> <td></td> </tr> </tbody> </table> <p>OPTIONAL END PLATE DETAIL FOR 38mm <math>\varnothing</math> ROD AND SMALLER</p>	ROD $\varnothing$	PLATE DIMENSIONS			WELD	A	B	C	19 (3/4")	60	13	10		25 (1")	75	16	13		32 (1 1/4")	95	19	17		38 (1 1/2")	115	22	21		45 (1 3/4")	135	25	24		51 (2")	150	29	27	
ROD $\varnothing$	PLATE DIMENSIONS			WELD																																			
	A	B	C																																				
19 (3/4")	60	13	10																																				
25 (1")	75	16	13																																				
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38 (1 1/2")	115	22	21																																				
45 (1 3/4")	135	25	24																																				
51 (2")	150	29	27																																				

**NOTES:**  
1. WELDING SHALL BE DONE IN ACCORDANCE WITH CSA W59 WITH WELD STRENGTH  $X_u=480$  MPa.  
2. STEEL ROD AND PLATE SHALL CONFORM TO REQUIREMENTS OF CSA G40.21 GRADE 300W.  
3. MINIMUM CONCRETE STRENGTH  $f_c=25$ MPa.



1 01 - FOUNDATION PLAN  
S100 1:100

NOTES:

- TOP OF SLAB DATUM ELEVATION IS AT GEODETIC ELEVATION 305.82m.
- WHERE CROSSED AND NOTED THE LOCAL DATUM FOR RAISED OR LOWERED AREAS ARE GIVEN RELATIVE TO THE LOWER FLOOR DATUM.
- EXCEPT AS CROSSED AND NOTED TOP OF FINISHED FLOOR IS 0 mm BELOW THE LOWER FLOOR DATUM.
- WHERE CROSSED AND NOTED SLAB DEPRESSIONS OR LOCALLY RAISED AREAS ARE GIVEN RELATIVE TO THE LOWER FLOOR DATUM.
- REFER TO THE GENERAL NOTES FOR DESIGN ULS AND SLS BEARING CAPACITIES.
- BEARING ELEVATIONS (UNDERSIDE OF FOOTING) ARE NOTED ON PLAN. THESE ARE APPROXIMATE AND MUST BE VERIFIED IN THE FIELD BY THE GEOTECHNICAL CONSULTANT.
- FOUND FOOTINGS AT A MINIMUM OF 1500mm BELOW FINISHED GRADE WHERE EXPOSED TO FROST.
- CENTRE ALL FOOTINGS AND CAPS ON THE GRID LINES UNLESS NOTED OTHERWISE.
- BOREHOLE LOCATIONS SHOWN ON PLAN ARE APPROXIMATE. ELEVATIONS OF EXISTING GRADE AND OF NATIVE SOIL ARE INDICATED AT EACH BOREHOLE.
- THE SITE CONTAINS BURIED TOPSOIL AND/OR FILL MATERIAL UNSUITABLE TO SUPPORT THE PROPOSED STRUCTURE. THE ELEVATIONS OF NATIVE SOIL AT BOREHOLES INDICATE COMPETENT SOIL UPON WHICH FOOTINGS MAY BE FOUNDED OR UPON WHICH ENGINEERED FILL MAY BE PLACED TO RAISE THE SUB-GRADE TO A SUITABLE FOUNDING ELEVATION. REFER TO THE GEOTECHNICAL REPORT FOR DETAILED SOIL INFORMATION.
- PROVIDE CONTROL JOINTS IN ALL FOUNDATION WALLS AS PER DETAIL 0315. COORDINATE CONTROL JOINT LOCATIONS WITH ARCHITECTURAL.

FOUNDATION SCHEDULE

MARK	DIMENSIONS (mm)			REINFORCEMENT	REMARKS
	LENGTH	WIDTH	DEPTH		
F1	900	900	300	3-15M BEW	
F2A	1200	1200	300	4-15M BEW, 4-15M TEW	
F3	1500	1500	350	6-15M BEW	
F3A	1500	1500	350	6-15M BEW, 6-15M TEW	
F4	1800	1800	400	6-20M BEW	
F4A	1800	1800	400	6-20M BEW, 6-15M TEW	
F5A	2100	2100	450	9-20M BEW, 9-15M TEW	
F6	2400	2400	500	12-20M BEW	
F6A	2400	2400	500	12-20M BEW, 12-15M TEW	
F7	2700	2700	550	10-25M BEW	
F7A	2700	2700	550	10-25M BEW, 10-15M TEW	
F8	3000	3000	600	12-25M BEW	
F10	3600	3600	600	16-25M BEW	
F12	4400	2200	700	14-25M BEW	
F13A	2700	2200	550	8-25M BEW, 8-15M TEW	
F14	2600	1700	550	12-20M BEW	
F15A	2100	1600	550	8-20M BEW, 8-15M TEW	
F16	1500	2700	450	9-20M BEW	
F16A	1500	2700	450	9-20M BEW, 9-15M TEW	
SF1	500	250		SEE TYPICAL DETAIL 0306	
SF2	600	250		SEE TYPICAL DETAIL 0306	
SF4	675	250		SEE TYPICAL DETAIL 0306	
SF5	15202	900	250	SEE TYPICAL DETAIL 0306	

CONCRETE PIER SCHEDULE

MARK	DIMENSION		REINFORCEMENT - VERTICAL	REINFORCEMENT - TIE	REMARKS
	DEPTH	WIDTH			
P1	500	500	8-20M	15M@300	
P2	700	700	8-25M	15M@300	

FOUNDATION WALL SCHEDULE

MARK	THICKNESS (mm)	HORIZ. REINF.	VERT. REINF.	REMARKS
FW400	400	15M@500 HEF	15M@500 HEF	15M@500 HEF, 15M@500 VIF FOR NOTCH
FW475	475	15M@400 HEF	15M@400 VEF	15M@400 HEF, 15M@400 VIF FOR NOTCH
FW700	700	3 LAYERS- 15M @ 200 H	3 LAYERS- 15M @ 200 V	SEE S/S502

Contractor must check and verify all dimensions on the job, and report any discrepancies to the Architect before proceeding with the work.

Do not scale this drawing.

ISSUE:  
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9	2022/05/30	ISSUED FOR STEEL RESTRUCTURING
8	2022/01/31	REVISED ISSUED FOR TENDER/ISSUED FOR BUILDING PERMIT
7	2021/09/28	ADDENDUM S2
6	2021/09/14	ADDENDUM S1
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1	2021/07/16	Issued for Class B Costing

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TEL 416-467-8151

PROJECT NAME:  
**NEW SAYERS FOOD STORE BURLEIGH STREET, APSLEY**

PROJECT ADDRESS:  
**132 Burleigh Street**

SEAL:

DRAWN: DM  
SCALE: As indicated  
CHECKED: IFM  
PROJECT NUMBER: 210112

SHEET TITLE:  
**FOUNDATION PLAN**

**S100**

Contractor must check and verify all dimensions on the job, and report any discrepancies to the Architect before proceeding with the work.

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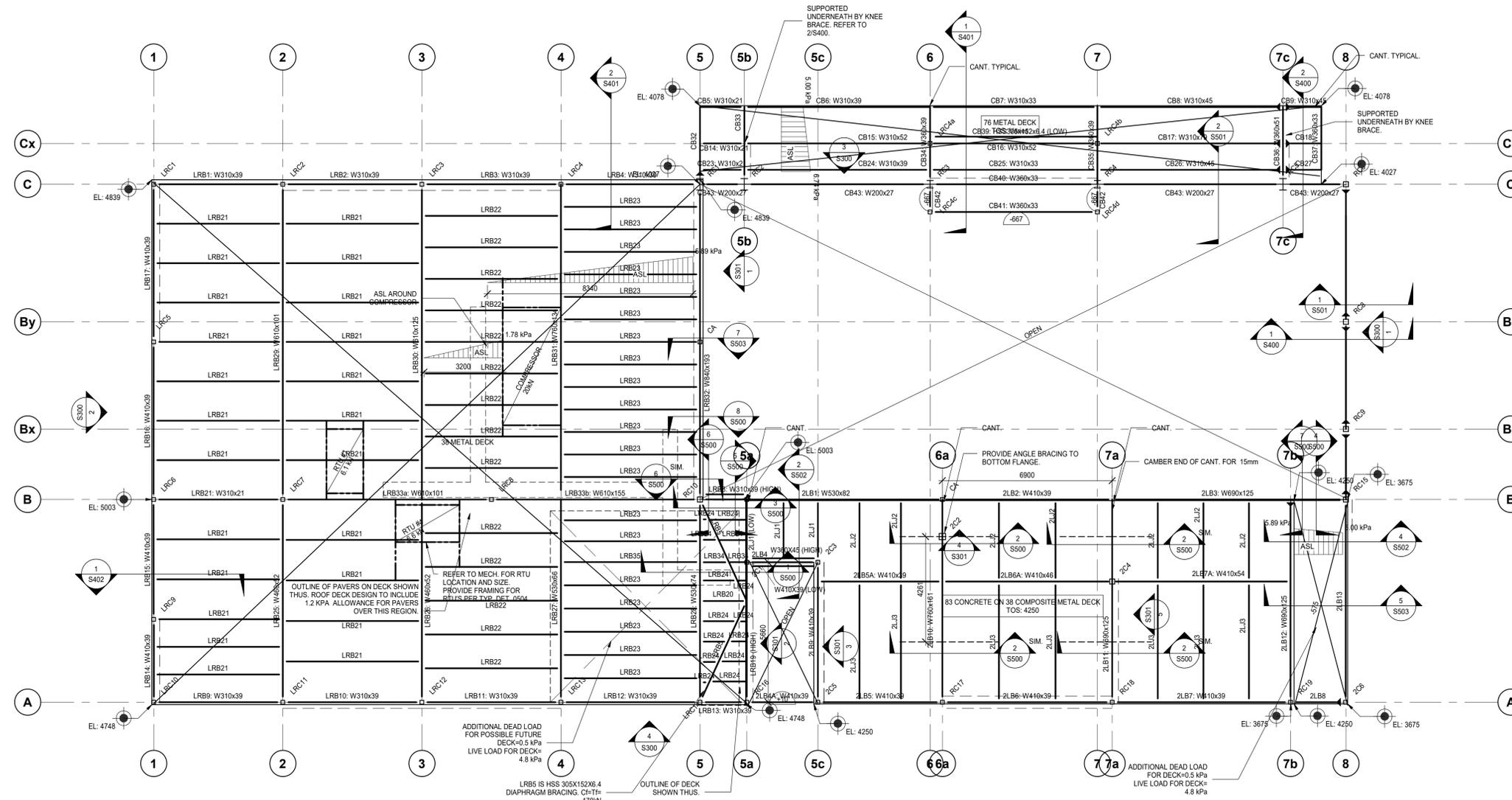
SEAL:

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SCALE:  
As indicated

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IFM  
PROJECT NUMBER:  
210112

SHEET TITLE:  
**SECOND & LOW ROOF FRAMING PLAN**

**S101**



02 - SECOND AND LOW ROOF FRAMING PLAN  
S101  
1:100

NOTES:

- SECOND FLOOR DATUM IS AT GEODETIC ELEVATION 310.07m EXCEPT AS CROSSED AND NOTED.
- WHERE CROSSED AND NOTED THE LOCAL DATUM FOR RAISED OR LOWERED AREAS ARE GIVEN RELATIVE TO THE GROUND FLOOR DATUM.
- EXCEPT AS CROSSED AND NOTED TOP OF FINISHED FLOOR IS 0 mm BELOW THE FLOOR DATUM.
- ROOF DATUM IS FROM THE GROUND FLOOR DATUM AS NOTED IN SPOT ELEVATIONS.
- THE ROOF DATUM REPRESENTS THE UNDERSIDE OF METAL DECK AT CORNERS. THE ROOF SLOPES. REFER TO ARCHITECTURAL DRAWINGS FOR THE SLOPES.
- TOP OF STEEL JOISTS AND STEEL BEAMS SUPPORTING THE SLAB ON METAL DECK AT LEVEL 2 ARE 121 mm BELOW THE FINISHED FLOOR UNLESS OTHERWISE NOTED.
- TOP OF STEEL BEAMS SUPPORTING JOISTS ARE 102 mm BELOW THE UNDERSIDE OF ROOF METAL DECK.
- WHERE NOTED, TOP OF BEAM IS GIVEN RELATIVE TO THE FINISHED FLOOR.
- SUPERIMPOSED LOADS USED IN THE DESIGN:  
LIVE LOAD ON 2ND FLR: 2.4 kPa  
DEAD LOADS ON 2ND FLR:  
PARTITIONS 1.0 kPa  
FLOOR FINISH 0.2 kPa  
SUSPENDED 0.25 kPa  
TOTAL 1.45 kPa  
SNOW + RAIN LOAD 2.88 kPa (PLUS SNOW ACCUMULATION SHOWN ON PLAN)  
DEAD LOADS ON ROOF AREAS:  
ROOFING & INSULATION 0.60 kPa  
SUSPENDED 0.25 kPa  
TOTAL 0.85 kPa
- ASSUMED SELF WEIGHT OF STRUCTURE USED IN THE DESIGN:  
76 METAL DECK 0.25 kPa  
38 METAL DECK 0.15 kPa  
83 CONC ON 38 DECK 2.45 kPa  
STEEL FRAMING 0.35 kPa

STEEL BEAM SCHEDULE

NOTES:

- LEFT AND RIGHT ENDS OF BEAMS ARE DEFINED BY THE ORIENTATION OF THE BEAM MARK ON PLAN.
- REACTIONS GIVEN ARE FACTORED FORCES. REACTIONS WITHIN BRACKETS DENOTE FACTORED UPLIFT FORCES.
- DESIGN CONNECTIONS FOR AXIAL COMPRESSION (C), AXIAL TENSION (T), STRONG-AXIS MOMENT (M), TORSIONAL MOMENT (Tm) OR OUT OF PLANE HORIZONTAL FORCE (H) SHOWN IN THE REMARKS COLUMN. IN ADDITION TO THE VERTICAL SHEAR PROVIDED IN THE REACTION COLUMN. THE (L) OR (R) SHOWN NEXT TO THE FORCE INDICATE THE LEFT OR RIGHT END, RESPECTIVELY.
- CAMBERS ARE IN mm. WHERE NO CAMBER IS INDICATED, REFER TO THE SPECIFICATION AND CSA S16.

STEEL BEAM SCHEDULE - LOW ROOF BEAM				
MARK	SIZE	REACTIONS		REMARKS
		LEFT END	RIGHT END	
LRB1	W310x39	35 kN	35 kN	Cf=30 kN Tf=30 kN
LRB2	W310x39	35 kN	35 kN	Cf=60 kN Tf=60 kN
LRB3	W310x39	35 kN	35 kN	Cf=160 kN Tf=160 kN
LRB4	W310x39	35 kN	35 kN	Cf=160 kN Tf=160 kN
LRB5	HSS305x152x6.4	20 kN	20 kN	Cf=170 kN Tf=170 kN
LRB8	W310x39	30 kN	30 kN	Tm(L)=10 kN-m Tm(R)=10 kN-m Cf=100 kN Tf=100 kN
LRB9	W310x39	35 kN	35 kN	Cf=30 kN Tf=30 kN
LRB10	W310x39	35 kN	35 kN	Cf=55 kN Tf=55 kN
LRB11	W310x39	35 kN	35 kN	Cf=150 kN Tf=150 kN
LRB12	W310x39	35 kN	35 kN	Cf=120 kN Tf=120 kN
LRB13	W310x39	35 kN	35 kN	Cf=85 kN Tf=85 kN
LRB14	W410x39	55 kN	55 kN	Cf=45 kN Tf=45 kN
LRB15	W410x39	55 kN	55 kN	Cf=30 kN Tf=30 kN
LRB16	W410x39	55 kN	55 kN	Cf=30 kN Tf=30 kN
LRB17	W410x39	55 kN	55 kN	Cf=55 kN Tf=55 kN
LRB19	W610x125	55 kN (55 kN)	260 kN	Cf=285 kN Tf=285 kN
LRB20	W310x21	20 kN	20 kN	Cf=10 kN Tf=10 kN
LRB21	W310x21	30 kN	30 kN	
LRB22	W310x21	40 kN	50 kN	
LRB23	W310x21	40 kN	45 kN	
LRB24	W310x21	20 kN	20 kN	
LRB25	W460x52	140 kN	145 kN	
LRB26	W460x52	145 kN	155 kN	
LRB27	W530x66	235 kN	235 kN	
LRB28	W530x74	255 kN	260 kN	Cf=165 kN Tf=165 kN
LRB29	W610x101	225 kN	220 kN	
LRB30	W610x125	295 kN	280 kN	
LRB31	W760x134	460 kN	445 kN	
LRB32	W840x193	315 kN	315 kN	Cf=165 kN Tf=165 kN
LRB33a	W610x101	160 kN	310 kN	Cf=85 kN Tf=85 kN
LRB33b	W610x155	475 kN	245 kN	Cf=95 kN Tf=95 kN
LRB34	W310x21	20 kN	20 kN	Cf=25 kN Tf=25 kN
LRB35	W310x21	40 kN	45 kN	Cf=25 kN Tf=25 kN

STEEL BEAM SCHEDULE - CANOPY BEAM				
MARK	SIZE	REACTIONS		REMARKS
		LEFT END	RIGHT END	
CB5	W310x21	20 kN	20 kN	
CB6	W310x39	55 kN	55 kN	
CB7	W310x39	50 kN	50 kN	
CB8	W310x45	60 kN	60 kN	M(R)=30 kN-m
CB9	W310x45	30 kN	30 kN	M(L)=30 kN-m
CB14	W310x21	35 kN	35 kN	
CB15	W310x52	100 kN	100 kN	
CB16	W310x52	100 kN	100 kN	
CB17	W310x79	100 kN	100 kN	M(R)=40 kN-m
CB18	W310x79	40 kN	40 kN	M(L)=40 kN-m
CB23	W310x21	20 kN	20 kN	
CB24	W310x39	55 kN	55 kN	
CB25	W310x45	60 kN	60 kN	
CB26	W310x45	60 kN	60 kN	M(R)=30 kN-m
CB27	W310x45	30 kN	30 kN	M(L)=30 kN-m
CB32	W360x79	230 kN	230 kN	H(L)=10 kN H(R)=10 kN M(L)=390 kN-m
CB33	W360x51	(45 kN)	285 kN	H(L)=10 kN H(R)=10 kN Tf=375 kN
CB34	W360x39	(75 kN)	430 kN	H(L)=10 kN H(R)=10 kN
CB35	W360x39	(75 kN)	430 kN	H(L)=10 kN H(R)=10 kN
CB36	W360x51	(-55 kN)	350 kN	H(L)=10 kN H(R)=10 kN Tf=545 kN
CB37	W360x39	15 kN	15 kN	
CB39	HSS305x152x6.4	25 kN	25 kN	H(L)=10 kN H(R)=10 kN
CB40	W360x33	40 kN	40 kN	
CB41	W360x33	20 kN	20 kN	
CB42	W200x15	15 kN	15 kN	Tf=10 kN
CB43	W200x27	15 kN	15 kN	

STEEL BEAM SCHEDULE - SECOND FLOOR BEAM				
MARK	SIZE	REACTIONS		REMARKS
		LEFT END	RIGHT END	
ZLB1	W530x82	100 kN	110 kN	Cf=95 kN Tf=95 kN
ZLB2	W410x39	75 kN	75 kN	Cf=95 kN Tf=95 kN
ZLB3	W690x125	135 kN	205 kN	H(R)=10 kN Tm(L)=10 kN-m Cf=95 kN Tf=95 kN
ZLB4	W410x39	70 kN	70 kN	
ZLB4A	W410x39	55 kN	55 kN	Cf=85 kN Tf=85 kN
ZLB5	W410x39	60 kN	60 kN	Cf=115 kN Tf=115 kN
ZLB5A	W410x39	95 kN	95 kN	
ZLB6	W410x39	80 kN	80 kN	Cf=155 kN Tf=155 kN
ZLB6A	W410x46	130 kN	130 kN	
ZLB7	W410x39	85 kN	85 kN	Cf=85 kN Tf=85 kN
ZLB7A	W410x54	140 kN	140 kN	
ZLB8	HSS305x152x6.4	85 kN	85 kN	H(L)=15 kN H(R)=15 kN M(R)=25 kN-m Tm(L)=15 kN-m Tm(R)=15 kN-m Cf=115 kN Tf=115 kN
ZLB9	W410x39	40 kN	110 kN	Cf=40 kN Tf=40 kN
ZLB10	W760x161	(-165 kN)	1490 kN	
ZLB11	W690x125	(-125 kN)	400 kN	Cf=180 kN Tf=180 kN
ZLB12	W690x125	145 kN	170 kN	M(R)=10 kN-m Cf=75 kN Tf=75 kN
ZLB13	HSS305x203x13	90 kN	90 kN	H(L)=25 kN H(R)=25 kN Tm(L)=25 kN-m Tm(R)=25 kN-m Cf=40 kN Tf=40 kN
ZLJ1	W310x21	20 kN	20 kN	
ZLJ2	W310x21	35 kN	35 kN	
ZLJ3	W310x21	55 kN	55 kN	

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PROJECT NAME:  
**NEW SAYERS FOOD STORE BURLEIGH STREET, APSLEY**

PROJECT ADDRESS:  
**132 Burleigh Street**

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DRAWN:  
**DM**

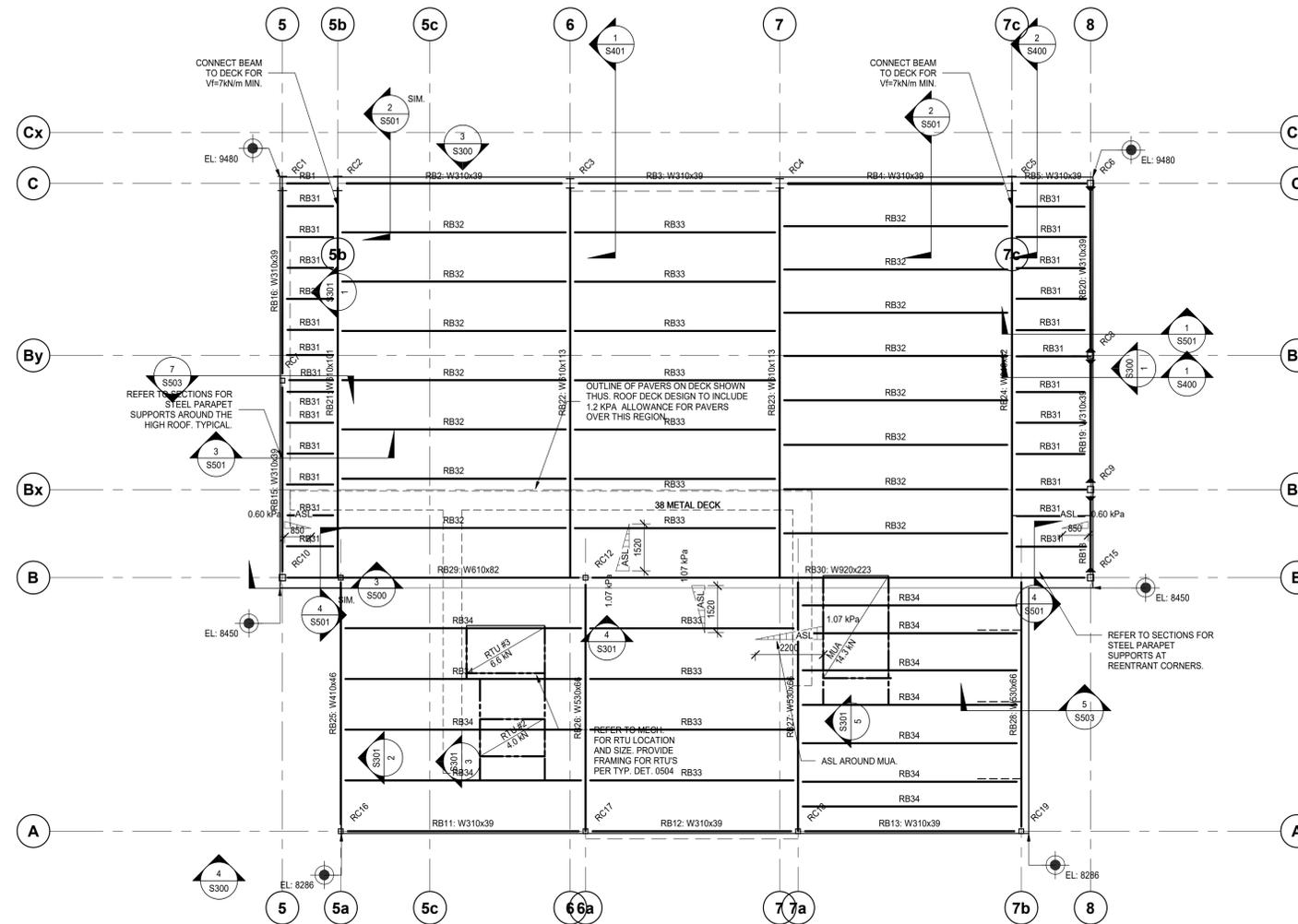
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**IFM**

SCALE:  
**As indicated**

PROJECT NUMBER:  
**210112**

SHEET TITLE:  
**HIGH ROOF FRAMING PLAN**

**S102**



1 03 - HIGH ROOF FRAMING PLAN

S102 1:100

NOTES:

- ROOF DATUM IS FROM THE GROUND FLOOR DATUM AS NOTED IN SPOT ELEVATIONS. THE ROOF DATUM REPRESENTS THE UNDERSIDE OF METAL DECK AT CORNERS. THE ROOF SLOPES, REFER TO ARCHITECTURAL DRAWINGS FOR THE SLOPES.
- TOP OF STEEL BEAMS ARE 102 mm BELOW THE UNDERSIDE OF ROOF DECK UNLESS NOTED THUS. WHERE NOTED, THE DIMENSION IS RELATIVE TO THE ROOF DATUM.
- SUPERIMPOSED LOADS USED IN THE DESIGN:  
SNOW + RAIN LOAD: 2.88 kPa (PLUS SNOW ACCUMULATION SHOWN ON PLAN)  
DEAD:  
ROOFING: 0.60 kPa  
SUSPENDED: 0.25 kPa
- SELF WEIGHT OF STRUCTURE USED IN THE DESIGN:  
METAL DECK: 0.15 kPa  
FRAMING: 0.35 kPa

STEEL BEAM SCHEDULE - HIGH ROOF

MARK	SIZE	REACTIONS		REMARKS
		LEFT END	RIGHT END	
RB1	W310x39	15 kN	15 kN	Cf=15 kN Tf=15 kN
RB2	W310x39	35 kN	35 kN	Cf=45 kN Tf=45 kN
RB3	W310x39	35 kN	35 kN	Cf=75 kN Tf=75 kN
RB4	W310x39	35 kN	35 kN	Cf=50 kN Tf=50 kN
RB5	W310x39	15 kN	15 kN	Cf=15 kN Tf=15 kN
RB11	W310x39	50 kN	50 kN	Cf=40 kN Tf=40 kN
RB12	W310x39	50 kN	50 kN	Cf=70 kN Tf=70 kN
RB13	W310x39	50 kN	50 kN	Cf=35 kN Tf=35 kN
RB15	W310x39	35 kN	35 kN	Cf=95 kN Tf=95 kN
RB16	W310x39	35 kN	35 kN	Cf=95 kN Tf=95 kN
RB18	W310x39	35 kN	35 kN	M(L)=40 kN-m M(R)=40 kN-m Cf=40 kN Tf=40 kN
RB19	W310x39	35 kN	35 kN	M(L)=25 kN-m M(R)=25 kN-m Cf=40 kN Tf=40 kN
RB20	W310x39	35 kN	35 kN	M(L)=30 kN-m M(R)=30 kN-m Cf=40 kN Tf=40 kN
RB21	W610x101	200 kN	190 kN	
RB22	W610x113	300 kN	290 kN	
RB23	W610x113	305 kN	300 kN	
RB24	W610x82	210 kN	205 kN	
RB25	W410x46	110 kN	115 kN	Cf=30 kN Tf=30 kN
RB26	W530x66	195 kN	205 kN	
RB27	W530x66	190 kN	220 kN	
RB28	W530x66	115 kN	130 kN	Cf=40 kN Tf=40 kN
RB29	W610x82	325 kN	405 kN	
RB30	W920x223	470 kN	595 kN	
RB31	W310x21	15 kN	15 kN	CAMBER 19mm
RB32	W310x28	40 kN	40 kN	
RB33	W310x28	45 kN	45 kN	
RB34	W310x28	55 kN	55 kN	

STEEL BEAM SCHEDULE NOTES:

- LEFT AND RIGHT ENDS OF BEAMS ARE DEFINED BY THE ORIENTATION OF THE BEAM MARK ON PLAN.
- REACTIONS GIVEN ARE FACTORED FORCES. REACTIONS WITHIN BRACKETS DENOTE FACTORED UPLIFT FORCES.
- DESIGN CONNECTIONS FOR AXIAL COMPRESSION (Cf), AXIAL TENSION (Tf), STRONG-AXIS MOMENT (M), TORSIONAL MOMENT (Tm) OR OUT OF PLANE HORIZONTAL FORCE (H) SHOWN IN THE REMARKS COLUMN. IN ADDITION TO THE VERTICAL SHEAR PROVIDED IN THE REACTION COLUMN, THE (L) OR (R) SHOWN NEXT TO THE FORCE INDICATE THE LEFT OR RIGHT END, RESPECTIVELY.
- CAMBERS ARE IN mm. WHERE NO CAMBER IS INDICATED, REFER TO THE SPECIFICATION AND CSA S16.

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PROJECT NAME:  
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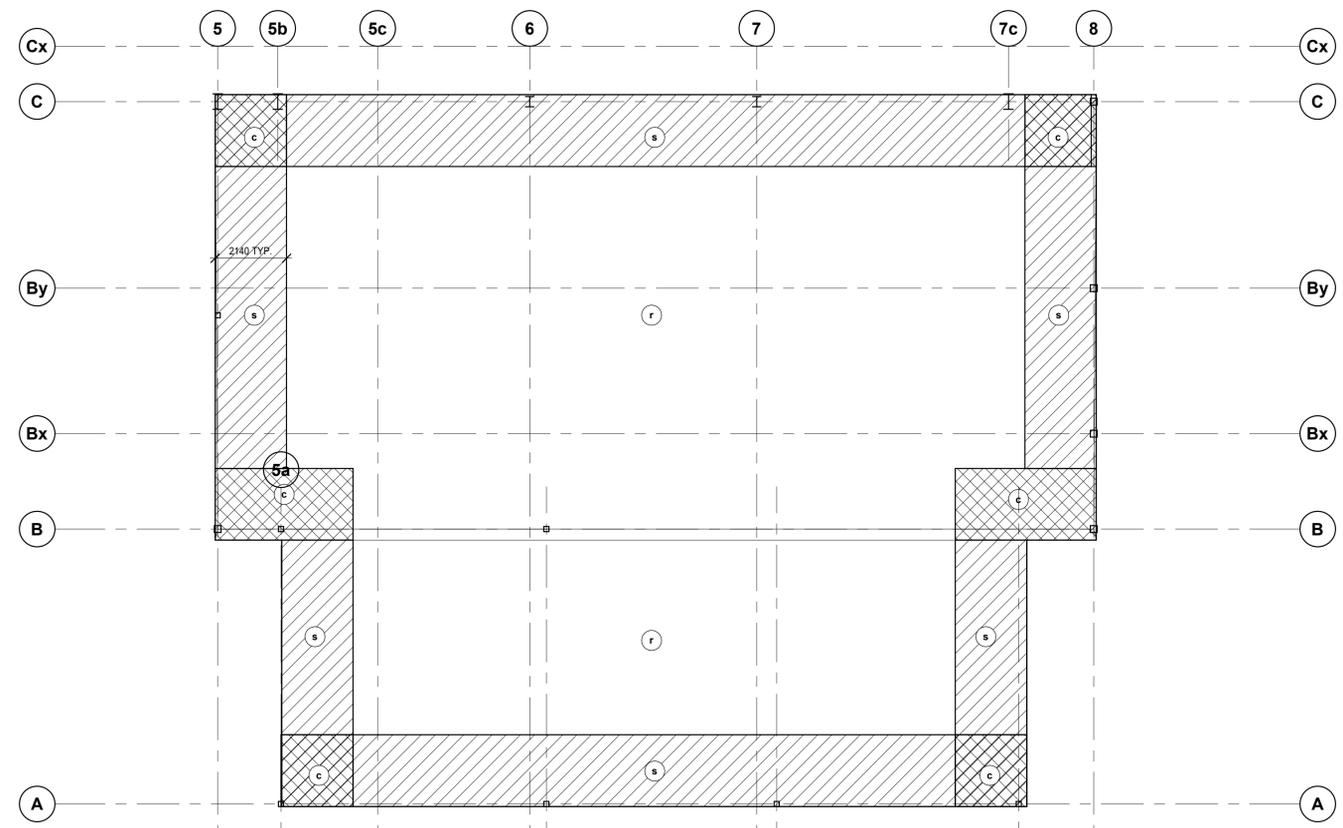
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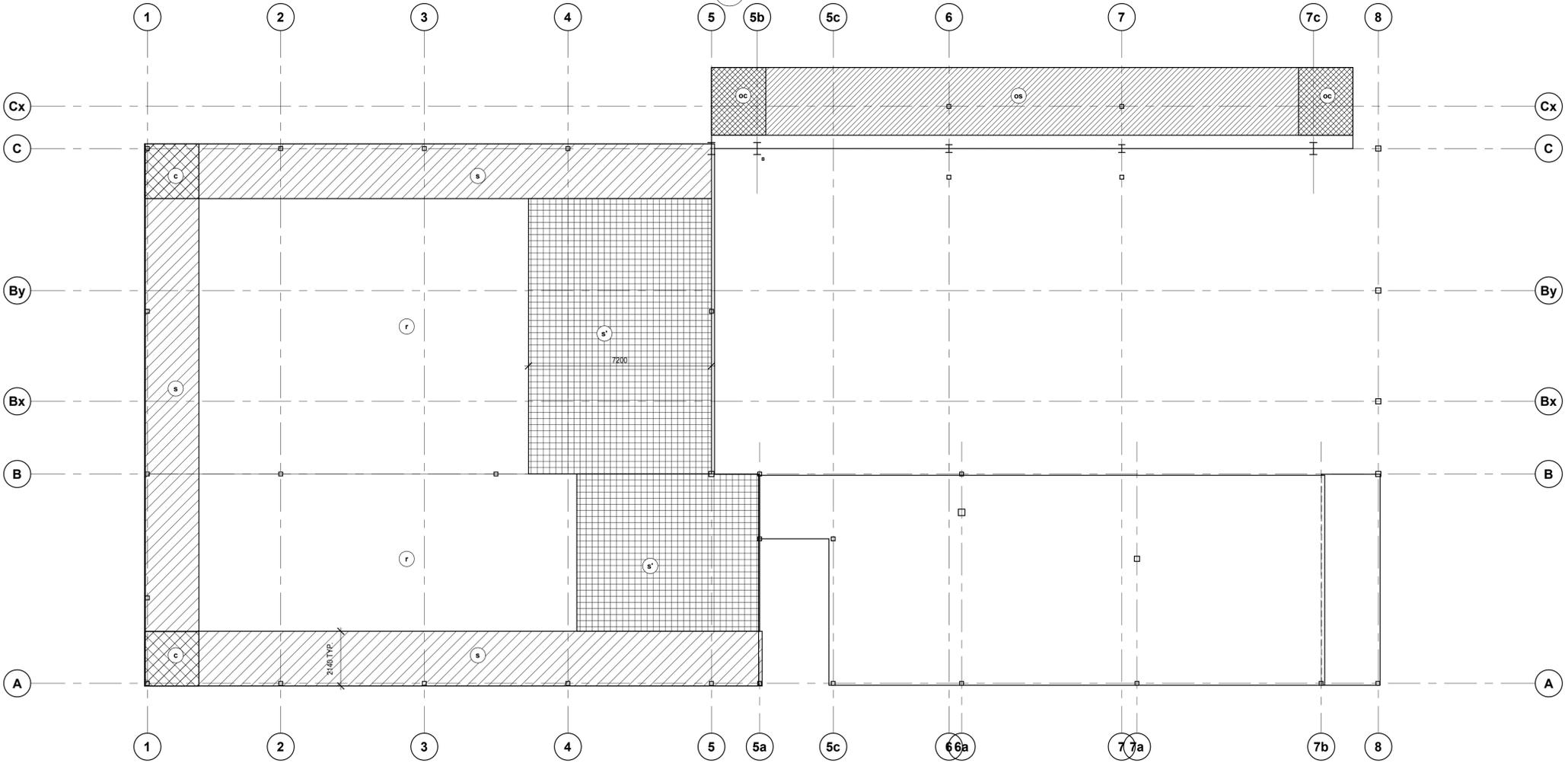
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SCALE: As indicated	PROJECT NUMBER: 210112

SHEET TITLE:  
**WIND UPLIFT DIAGRAMS**

**S103**



1 WIND UPLIFT - HIGH ROOF  
S103 1:100



2 WIND UPLIFT - LOW ROOF  
S103 1:100

**WIND UPLIFT DIAGRAM**

NOTES:

- PRESSURES SHOWN ARE UNFACTORED DESIGN UPLIFT PRESSURES IN kPa FOR THE DESIGN OF JOISTS AND METAL DECK AND THEIR CONNECTIONS.
- UPLIFT PRESSURES SHOWN HAVE BEEN REDUCED FOR THE EFFECT OF DEAD LOADS.
- PRESSURES ARE INCLUSIVE OF INTERNAL PRESSURE.

NET UPLIFT PRESSURES (kPa):						
ZONE	r	s	c	s'	os	oc
JOISTS	0.31	0.47	0.55	0.38	0.54	1.03
DECK	0.40	0.53	1.18	0.45	0.56	1.05

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PROJECT NAME:  
**NEW SAYERS FOOD STORE BURLEIGH STREET, APSLEY**

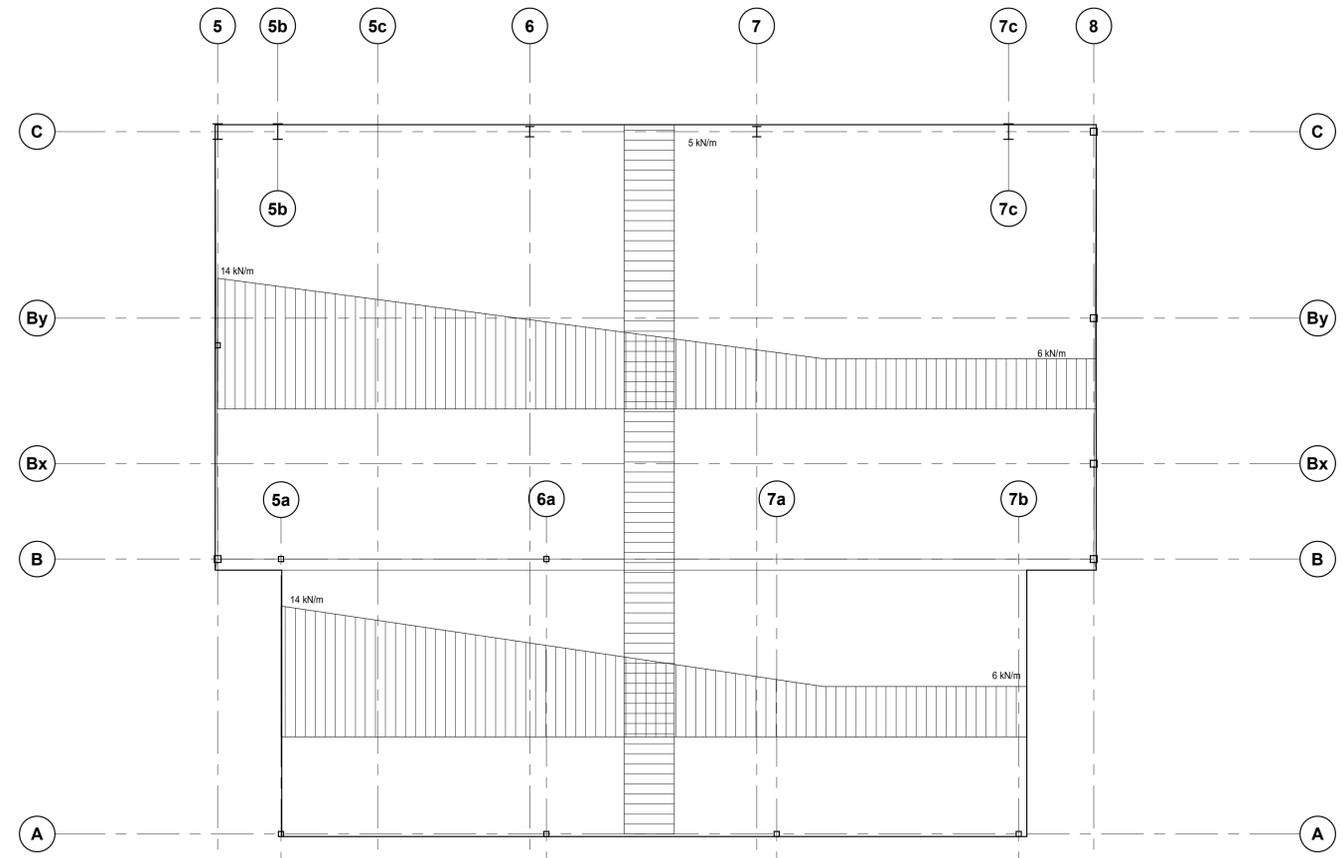
PROJECT ADDRESS:  
**132 Burleigh Street**

SEAL:

DRAWN: DM	CHECKED: IFM
SCALE: As indicated	PROJECT NUMBER: 210112

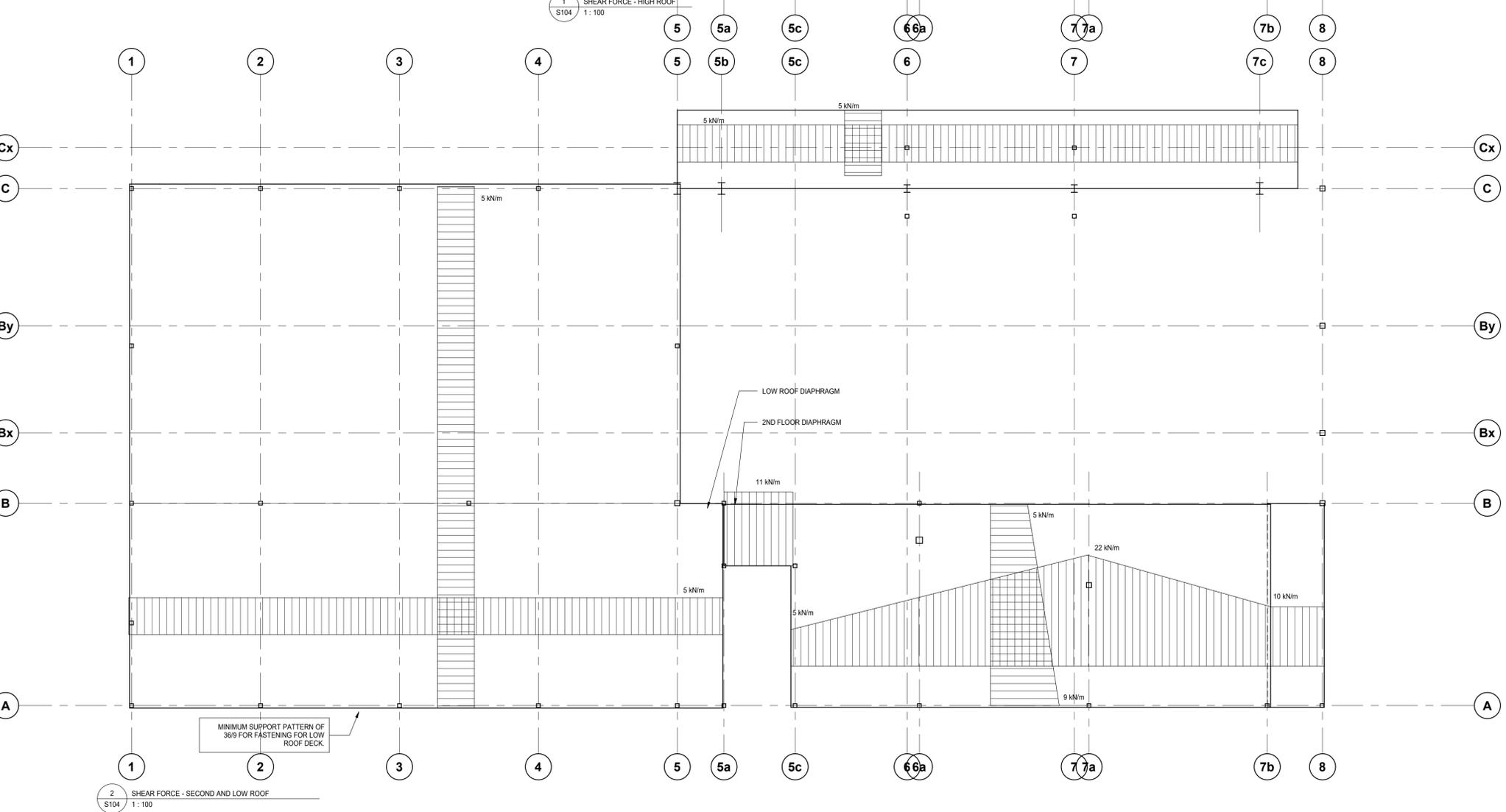
SHEET TITLE:  
**SHEAR FORCE DIAGRAMS**

**S104**



**ROOF SHEAR FORCE DIAGRAM**

NOTES:  
1. SHEAR FORCES SHOWN ARE FACTORED (ULS DESIGN FORCES IN kN/m FOR THE DESIGN OF METAL DECK AND ASSOCIATED CONNECTIONS).  
2. MECHANICAL FASTENERS HAVE BEEN ASSUMED WITH AN R<sub>o</sub>R<sub>w</sub> = 1.95. IF WELDED CONNECTIONS ARE TO BE USED, THE FACTORED FORCES MUST BE INCREASED 1.5 TIMES BASED ON AN R<sub>o</sub>R<sub>w</sub> = 1.3.



COLUMN SCHEDULE																				
HIGH PARAPET																				HIGH PARAPET
10000																				10000
LOW PARAPET																				LOW PARAPET
5500 LEVEL 2																				5500 LEVEL 2
4250																				4250
GROUND LEVEL																				GROUND LEVEL
0																				0
UIS FOOTINGS																				UIS FOOTINGS
-1500																				-1500
COLUMN LOCATIONS	LRC10	LRC9	LRC11	LRC12	LRC13	LRC14	RC16	2C5	RC17	RC18	RC19	2C6	LRC6	LRC7	LRC8	RC10	RC20	2C1	2C3	RC12

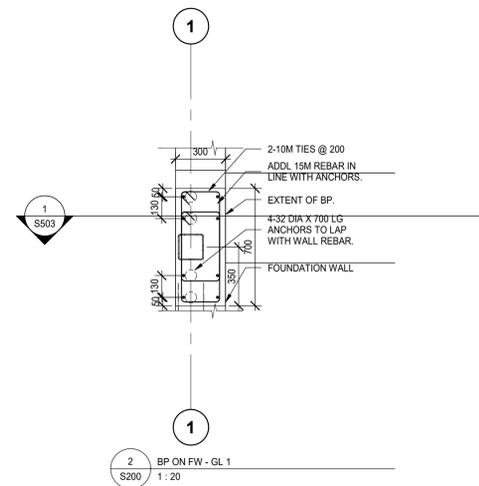
COLUMN SCHEDULE																				
HIGH PARAPET																				HIGH PARAPET
10000																				10000
LOW PARAPET																				LOW PARAPET
5500 LEVEL 2																				5500 LEVEL 2
4250																				4250
GROUND LEVEL																				GROUND LEVEL
0																				0
UIS FOOTINGS																				UIS FOOTINGS
-1500																				-1500
COLUMN LOCATIONS	2C2	2C4	RC15	LRC1	LRC2	LRC3	LRC4	RC1	RC2	RC3	LRC4c	RC4	LRC4d	RC5	RC6	RC9	LRC5	RC7	RC8	LRC4a

COLUMN SCHEDULE	
HIGH PARAPET	HIGH PARAPET
10000	10000
LOW PARAPET	LOW PARAPET
5500 LEVEL 2	5500 LEVEL 2
4250	4250
GROUND LEVEL	GROUND LEVEL
0	0
UIS FOOTINGS	UIS FOOTINGS
-1500	-1500
	LRC4b

**STEEL COLUMN SCHEDULE**

**NOTES:**

- WHERE NOTED WITH AN ASTERISK (\*) PROVIDE HEADED ANCHOR RODS; REFER TO TYPICAL DETAIL 0516. NOTE: PROVIDE 6.4mm PLATE WASHERS FOR ALL ANCHOR BOLTS LARGER THAN 25mm DIA. WITH HOLE TOLERANCE OF 1.6mm. WELD TO BASEPLATE AND ANCHOR BOLT FOR CAPACITY ONCE STEEL IS ERECTED AND PLUMB.
- CENTRE COLUMNS, CAPS AND FOOTINGS ON GRIDS UNLESS NOTED OTHERWISE.
- COLUMNS AND PIERS ARE ORIENTED AS SHOWN ON PLAN.
- COLUMN FORCES INDICATED ARE FACTORED IN KN AND BENDING MOMENTS (IF APPLICABLE) ARE FACTORED IN kN-m, UNLESS NOTED OTHERWISE.
- UPLIFT (TENSION) FORCES ARE PRESENTED IN BRACKETS BESIDE THE ASSOCIATED COMPRESSION FORCE, IF APPLICABLE. UPLIFT FORCES ARE FACTORED IN kN UNLESS NOTED OTHERWISE.
- WHERE MOMENTS OR SHEAR FORCES ARE PRESENTED SINGULARLY, THE MOMENT/SHEAR FORCE IS IN THE STRONG DIRECTION. IF THE COLUMN IS SQUARE, THE MOMENT/SHEAR FORCE IS IN BOTH DIRECTIONS UNLESS NOTED OTHERWISE.
- WHERE MOMENTS OR SHEARS ARE PRESENTED ABOUT TWO AXES, THE FIRST MOMENT/SHEAR FORCE IS IN THE STRONG DIRECTION AND THE SECOND IN THE WEAK DIRECTION. IF THE COLUMN IS SQUARE, THE FIRST MOMENT/SHEAR FORCE IS PARALLEL TO THE NORTH-SOUTH DIRECTION.
- REFER TO TYPICAL DETAIL 0303 UNLESS NOTED OTHERWISE.
- PROVIDE 4-19 DIAM. HOOKED ANCHOR BOLTS AS PER TYPICAL DETAIL 0303 UNLESS NOTED OTHERWISE.
- WHERE HEADED ANCHOR RODS ARE SPECIFIED REFER TO TYPICAL DETAIL 0516.



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PROJECT NAME:  
**NEW SAYERS FOOD STORE BURLEIGH STREET, APSLEY**

PROJECT ADDRESS:  
**132 Burleigh Street**

SEAL:

DRAWN: DM	CHECKED: IFM
SCALE: As indicated	PROJECT NUMBER: 210112

SHEET TITLE:  
**COLUMN SCHEDULE**

**S200**

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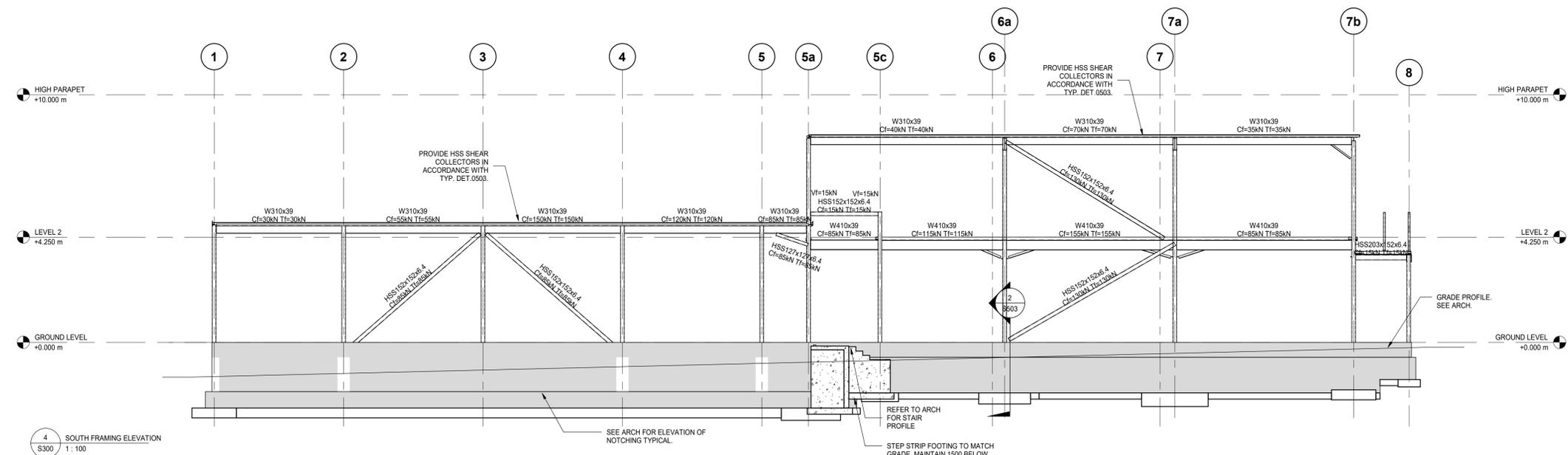
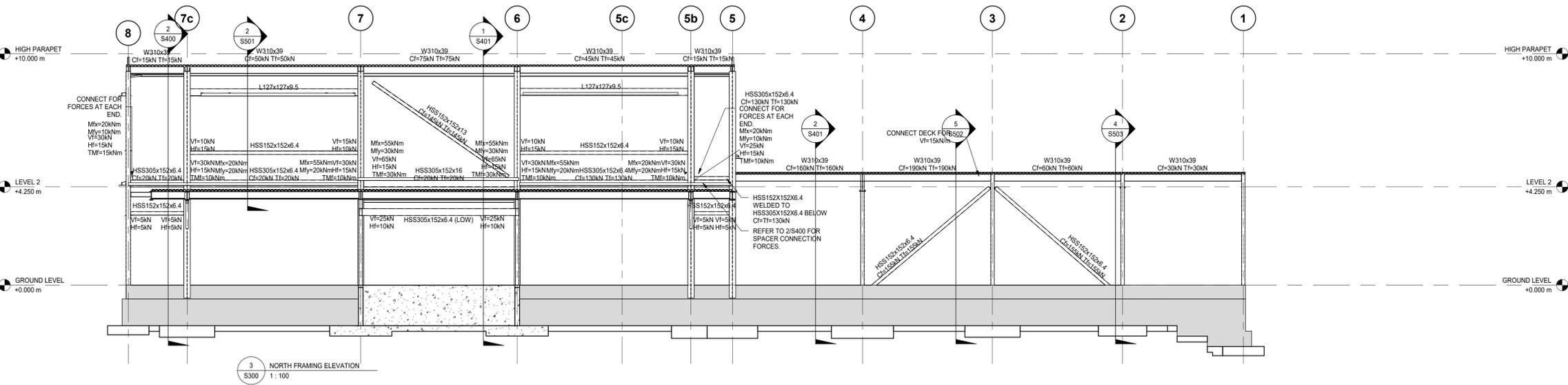
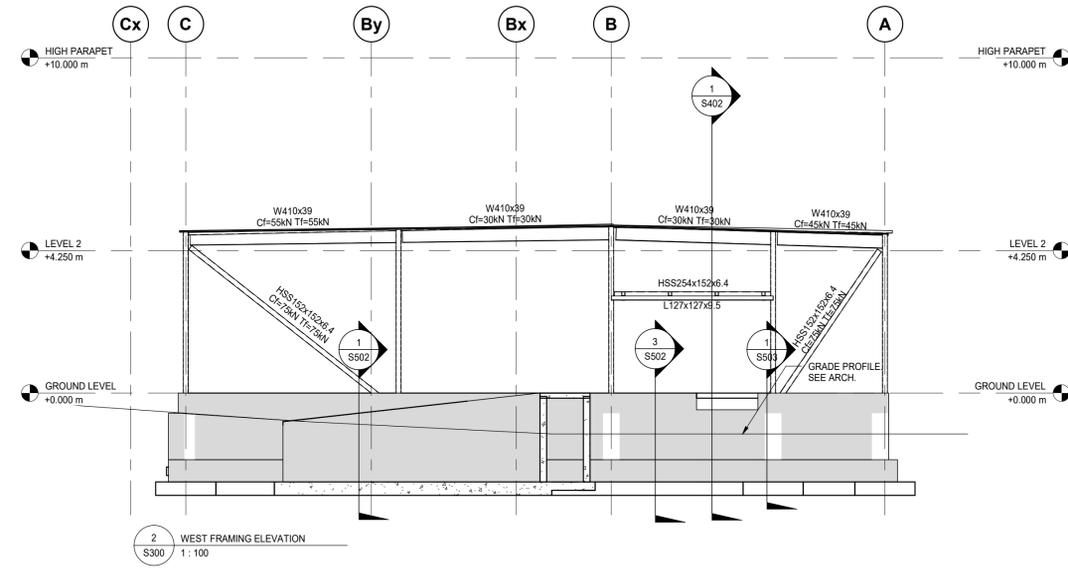
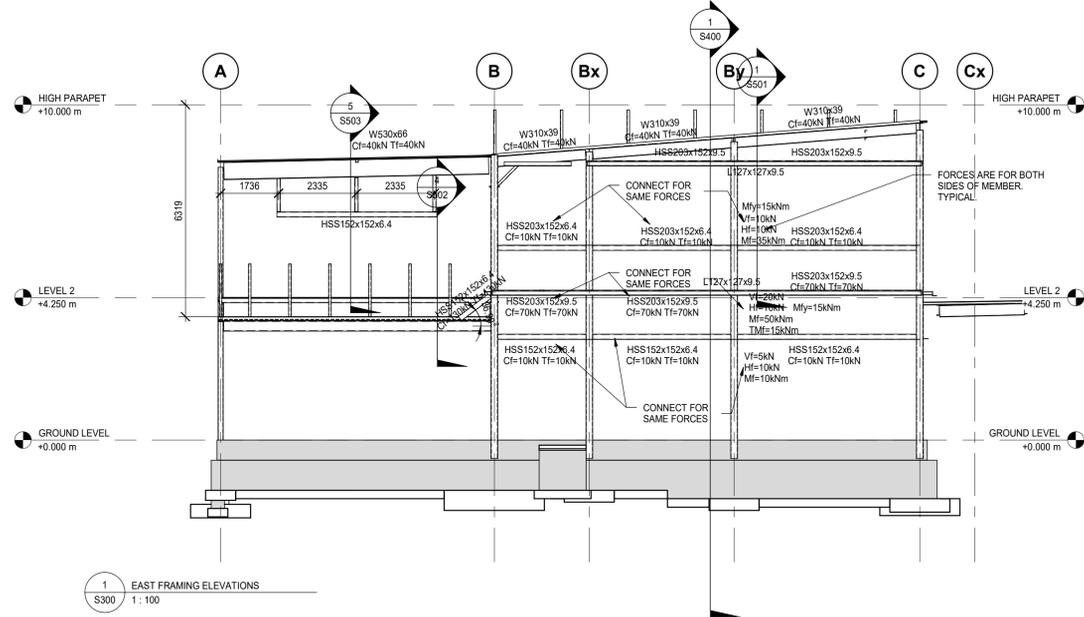
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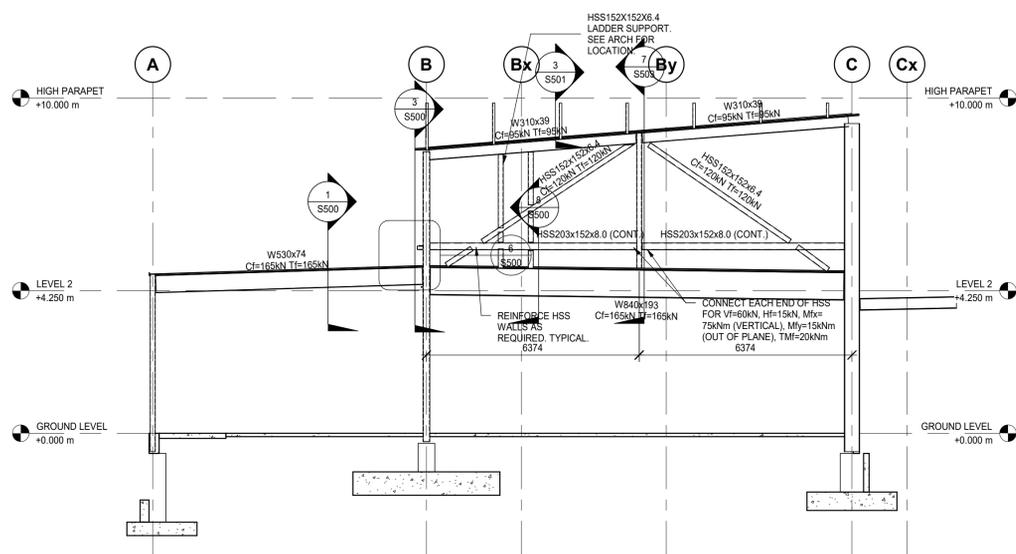
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SCALE:  
1 : 100

CHECKED:  
IFM  
PROJECT NUMBER:  
210112

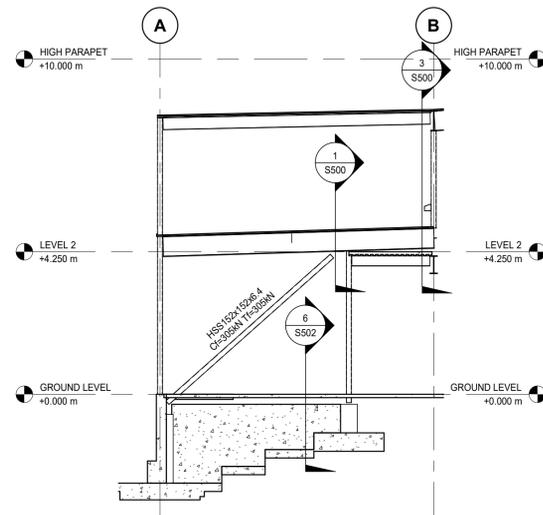
SHEET TITLE:  
**FRAMING ELEVATIONS**

**S300**

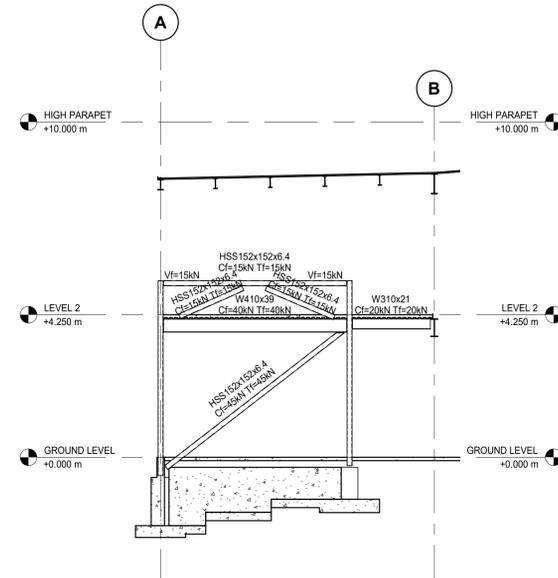




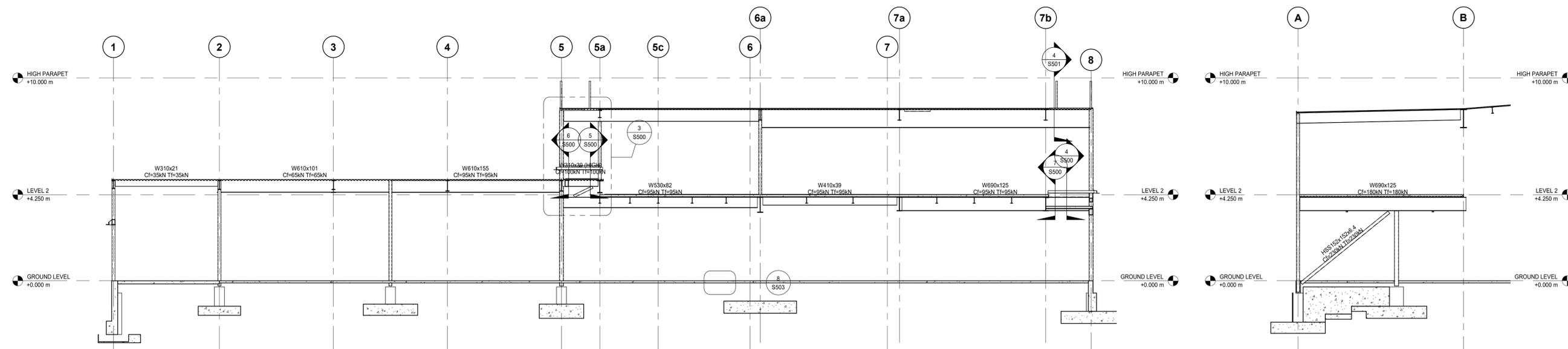
1 HIGH ROOF BRACING  
S301 1:100



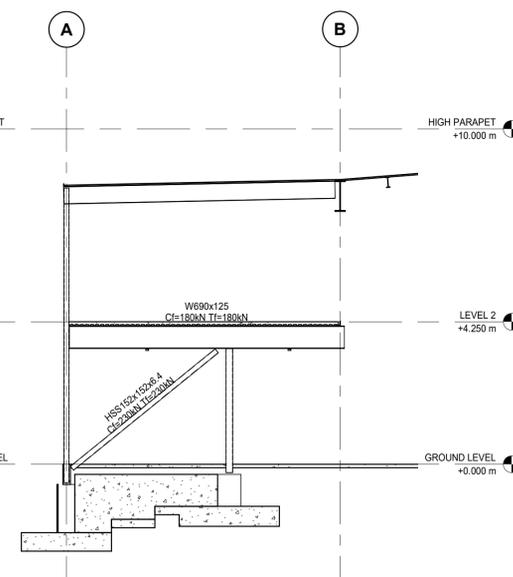
2 FRAMING BRACING NEAR GL 5a  
S301 1:100



3 FRAMING BRACING ON GL 6b  
S301 1:100



4 LINE B  
S301 1:100



5 BRACING NEAR GL 7  
S301 1:100

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DRAWN:  
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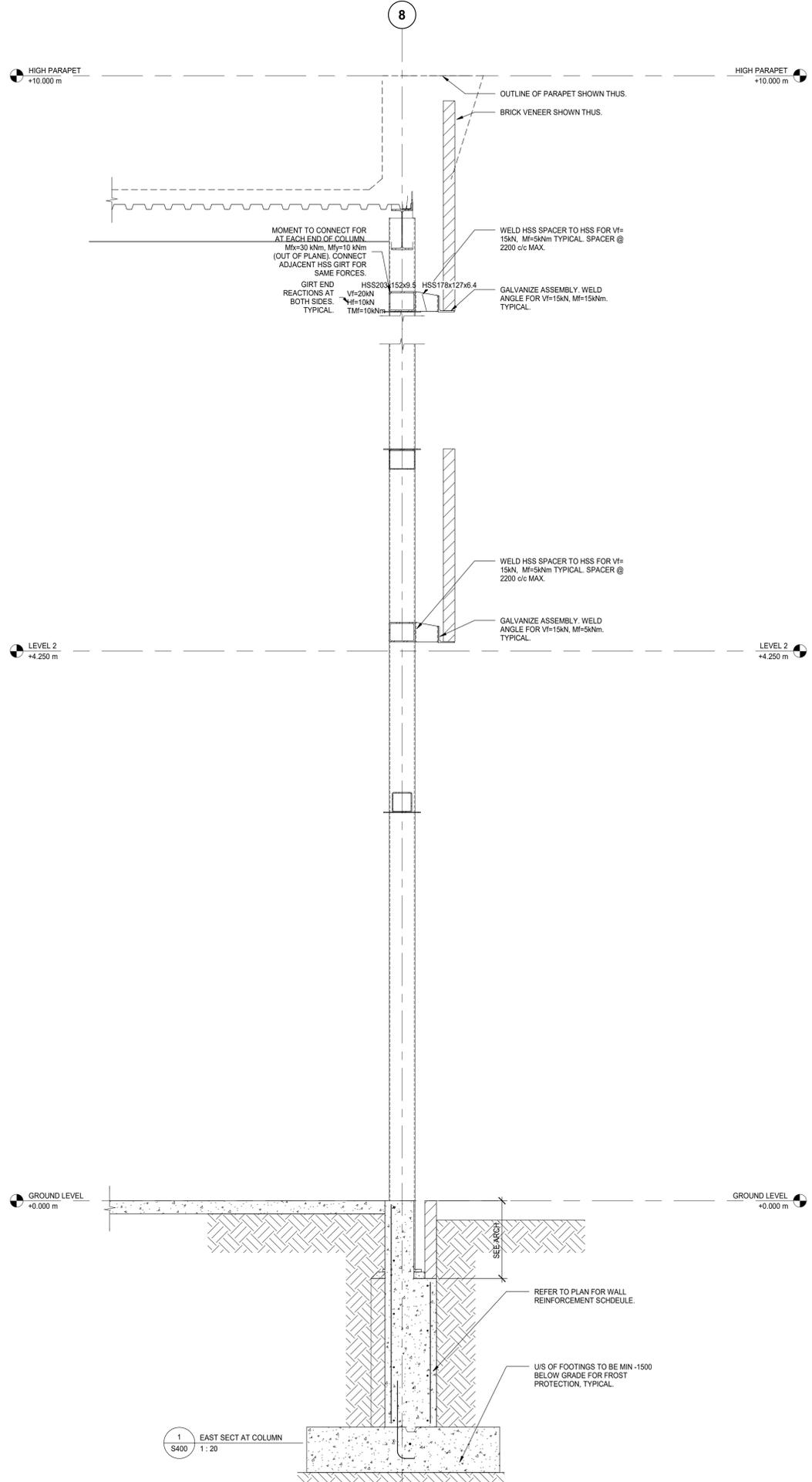
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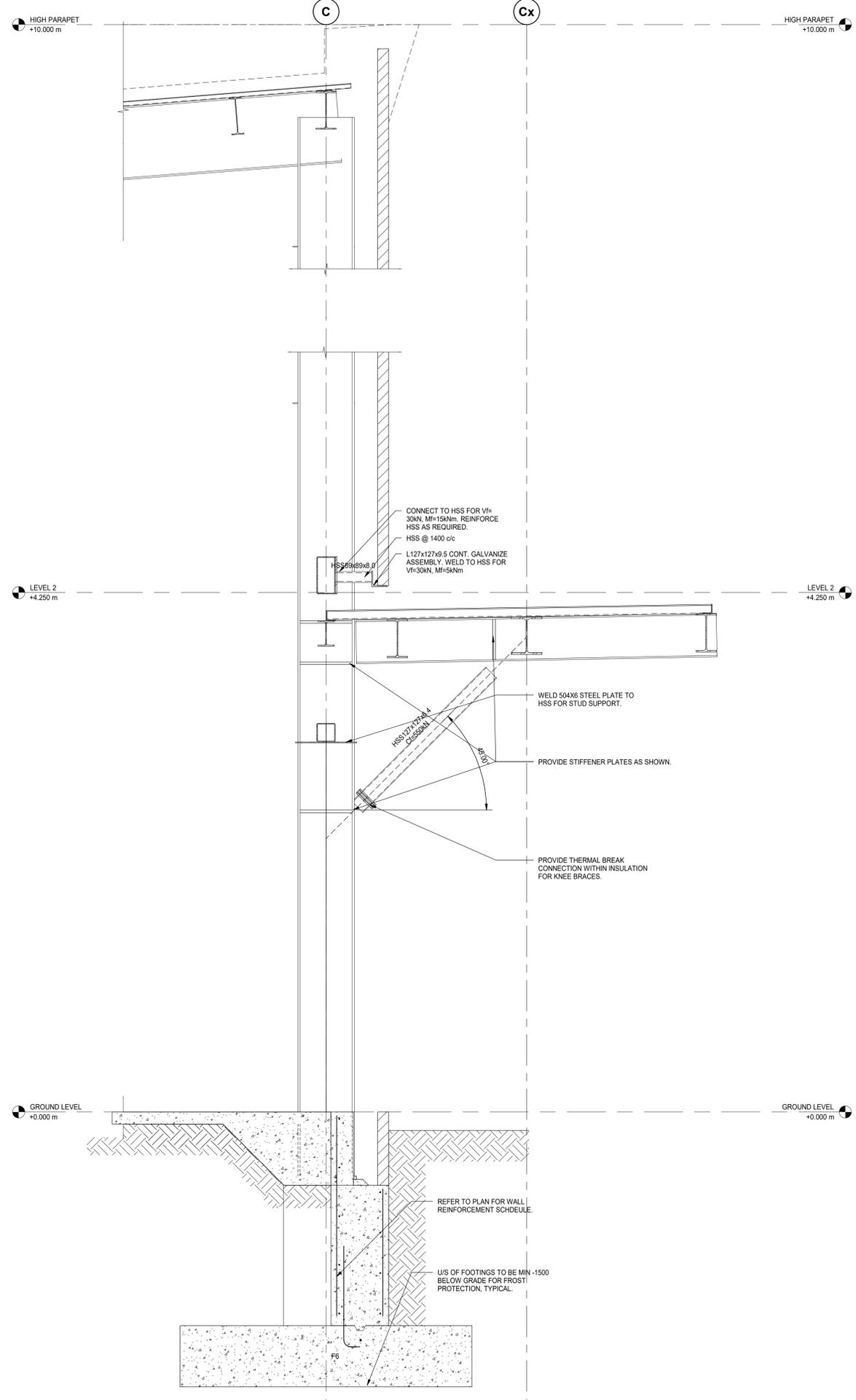
PROJECT NUMBER:  
210112

SHEET TITLE:  
**FRAMING ELEVATIONS**

# S301



1 EAST SECT AT COLUMN  
S400 1:20



2 NORTH SECT AT COL  
S400 1:20

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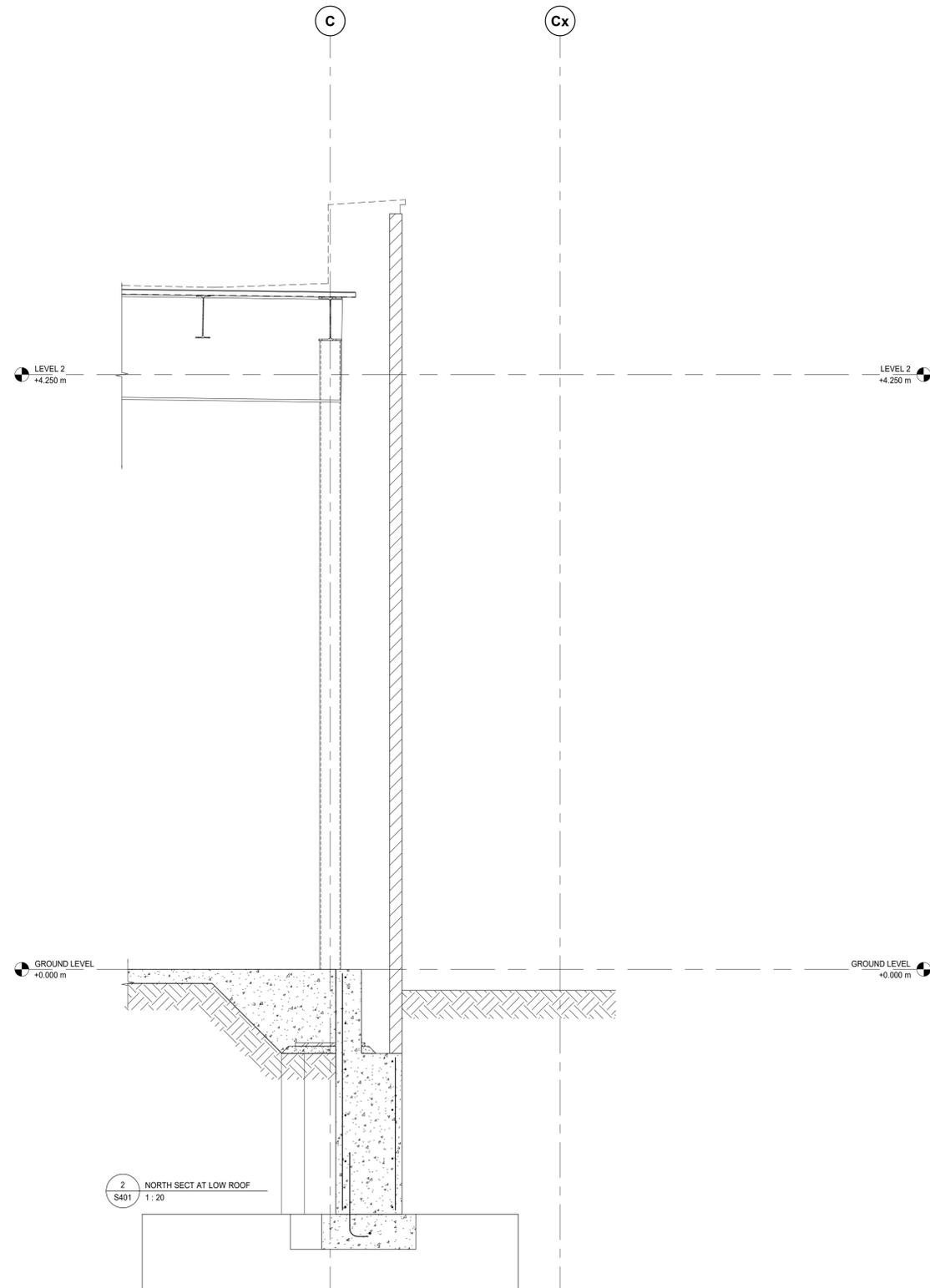
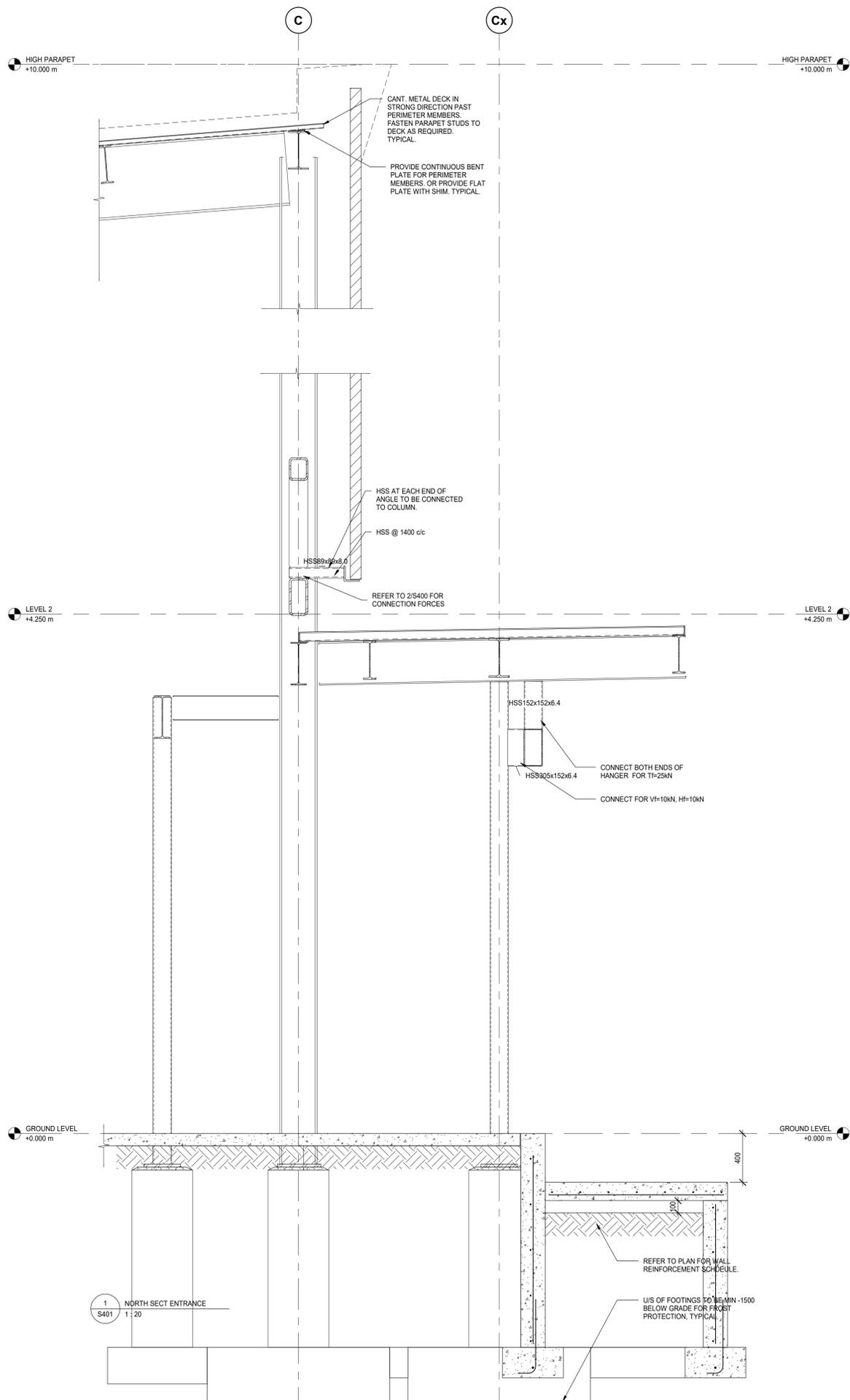
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SHEET TITLE:  
**BUILDING SECTIONS**

# S400



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SHEET TITLE:  
**BUILDING SECTIONS**

**S401**

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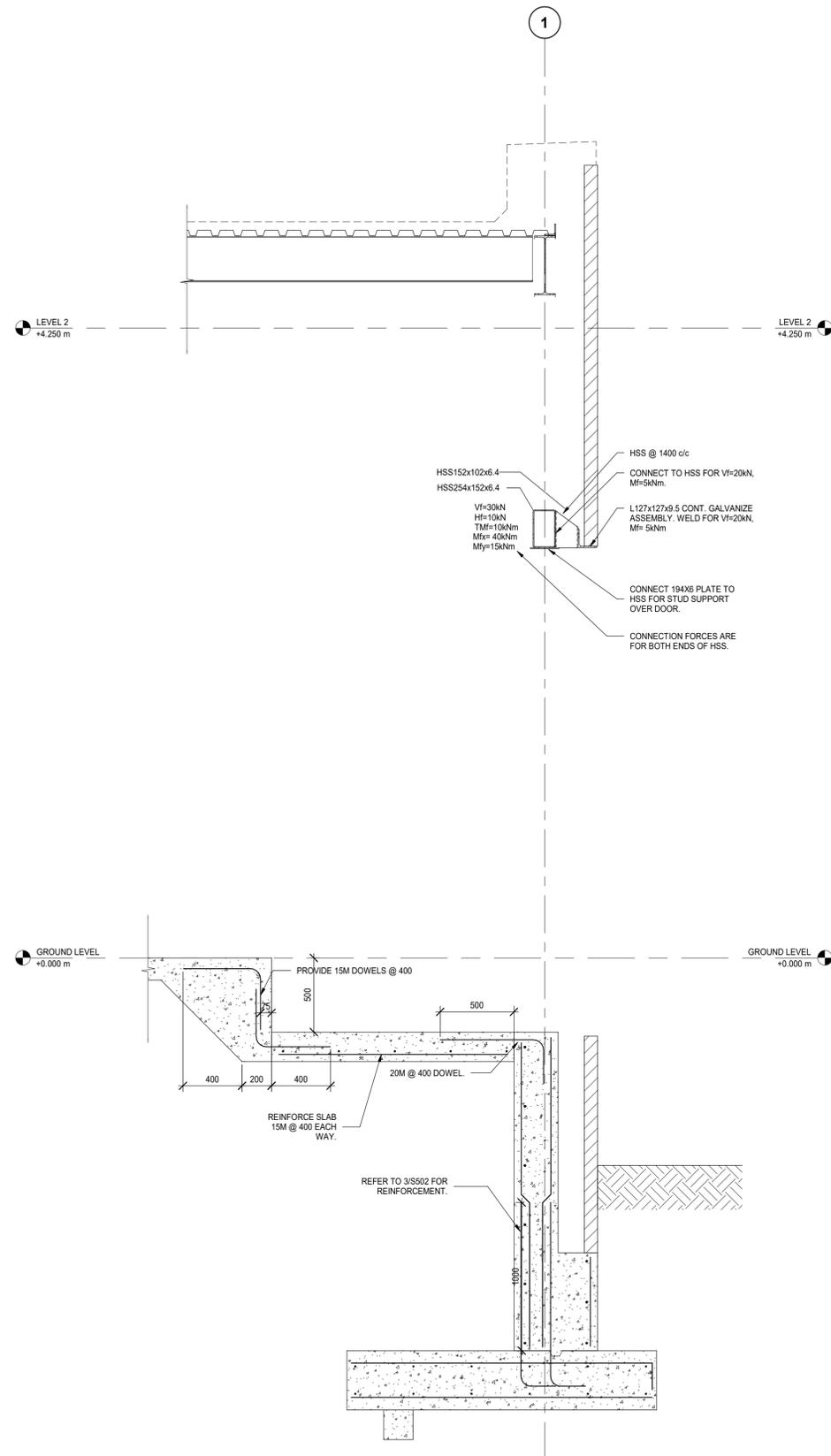
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SCALE: <b>1 : 20</b>	PROJECT NUMBER: <b>210112</b>

SHEET TITLE:  
**BUILDING SECTIONS**

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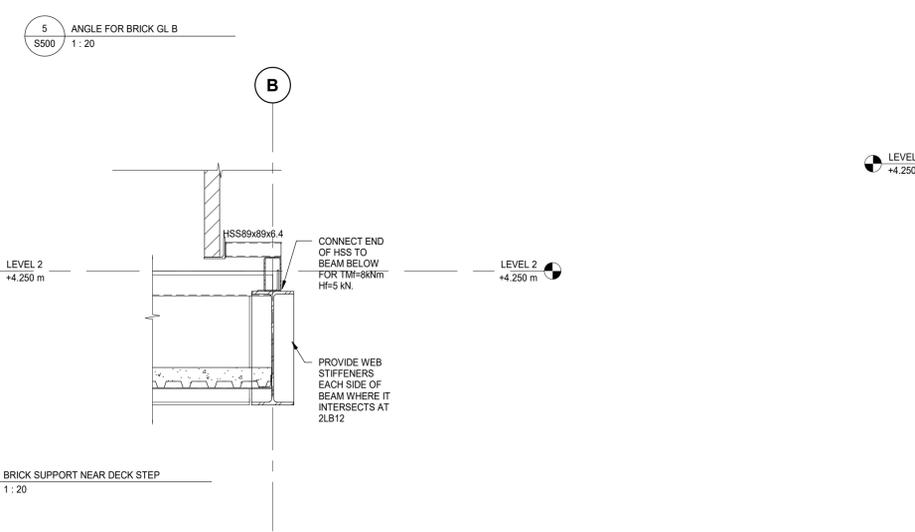
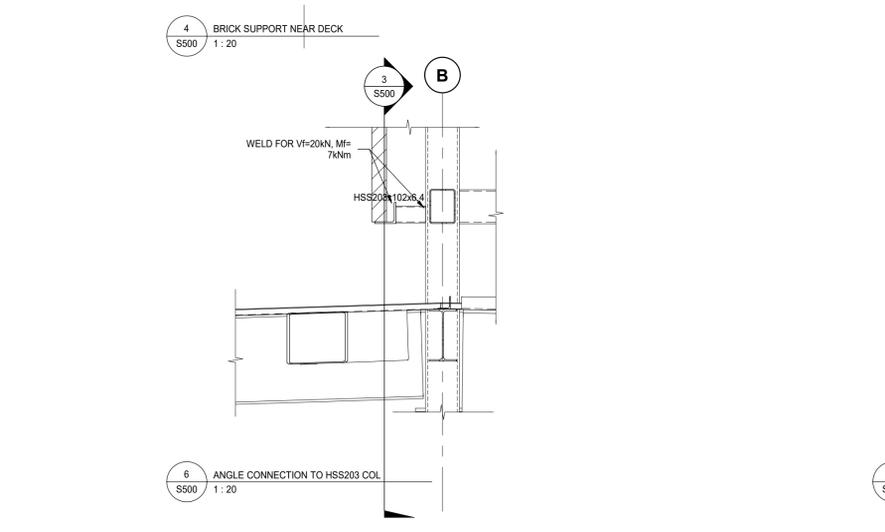
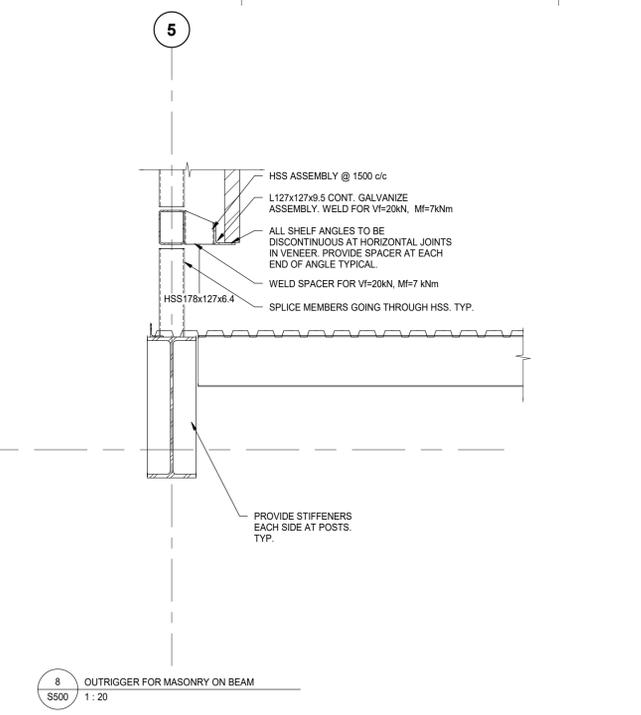
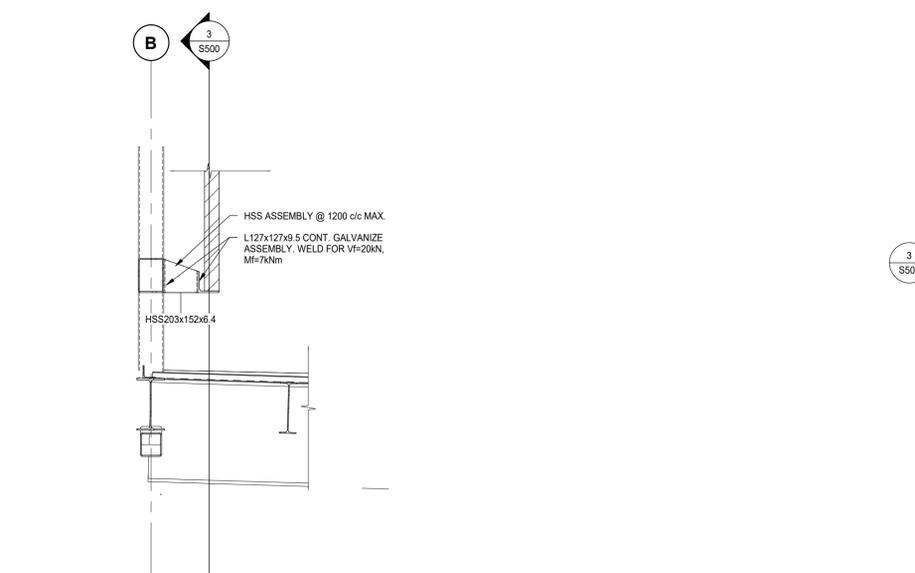
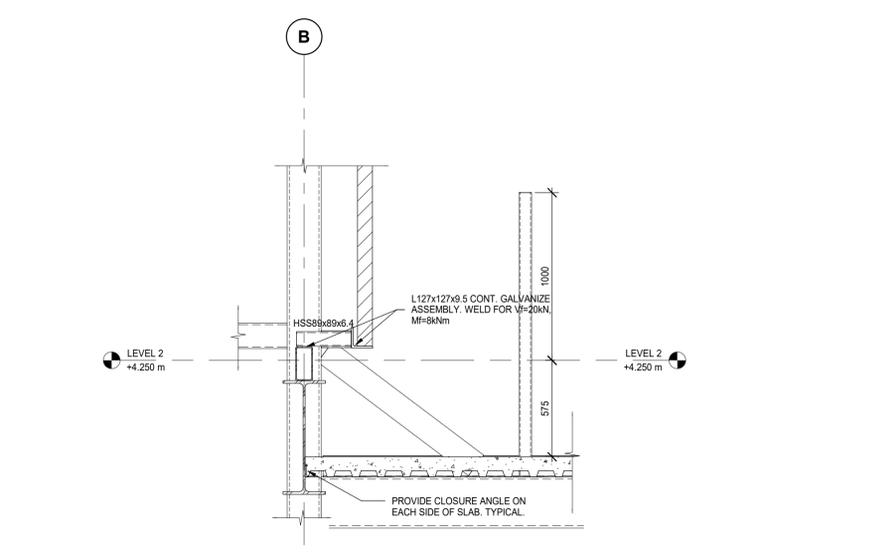
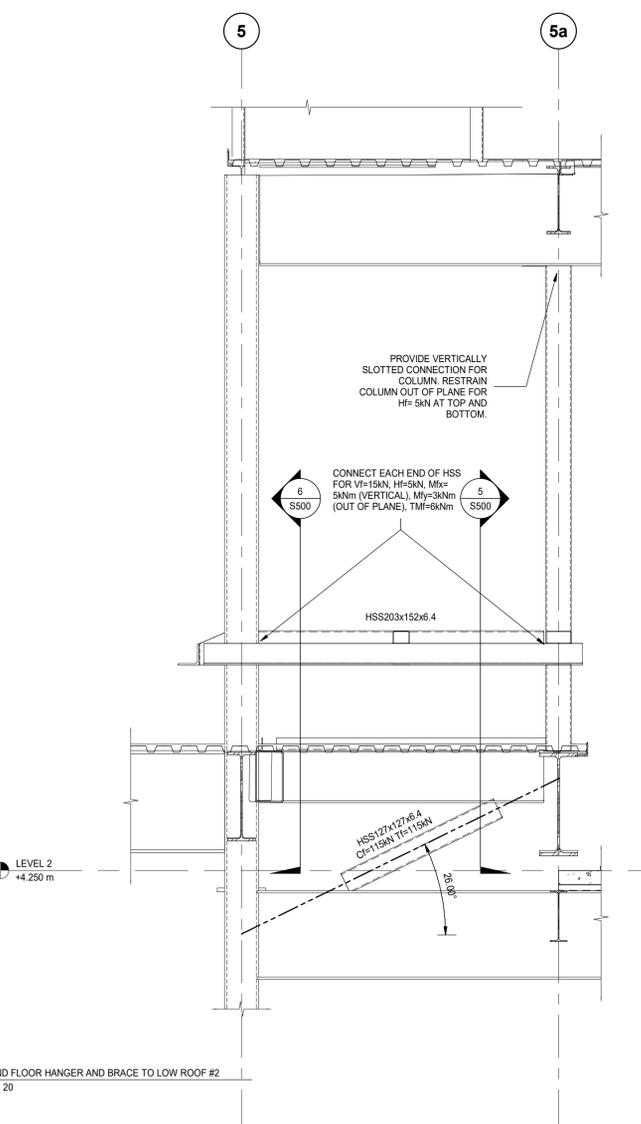
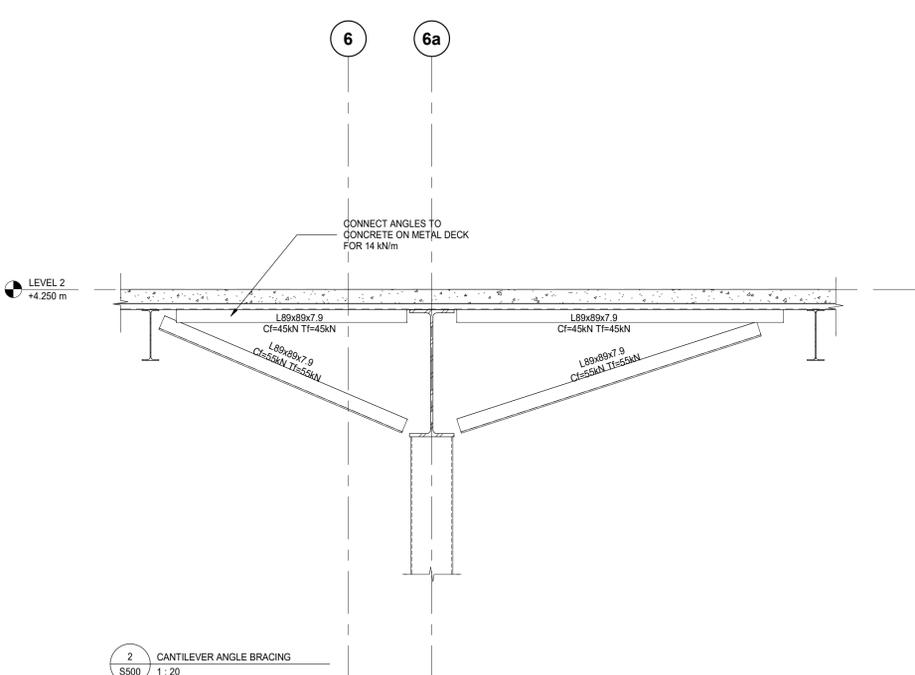
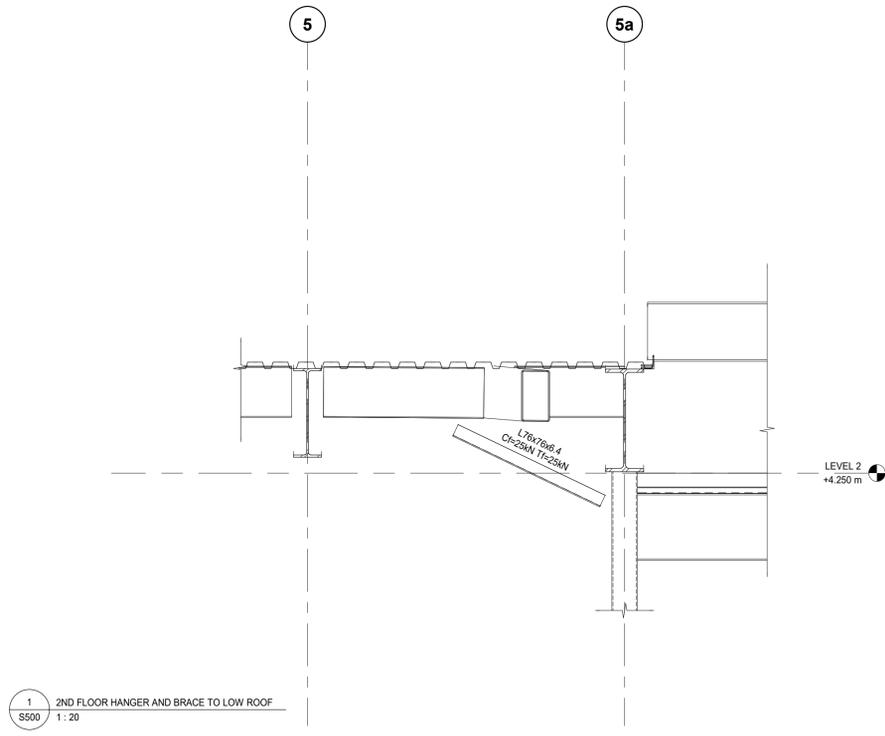
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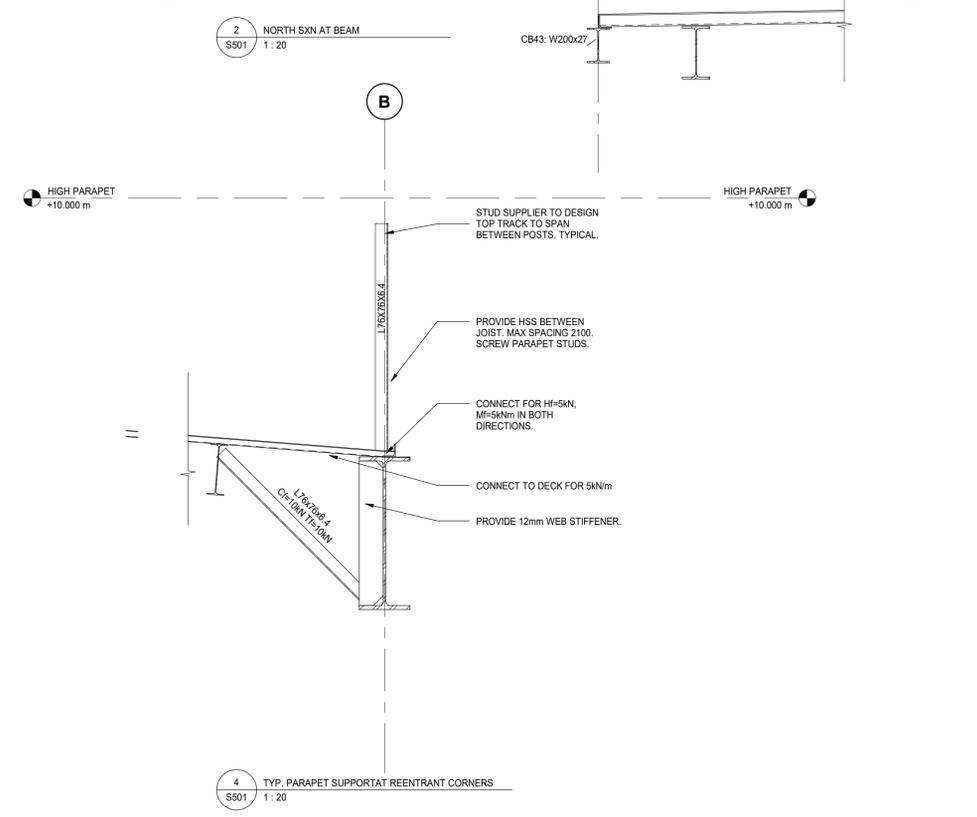
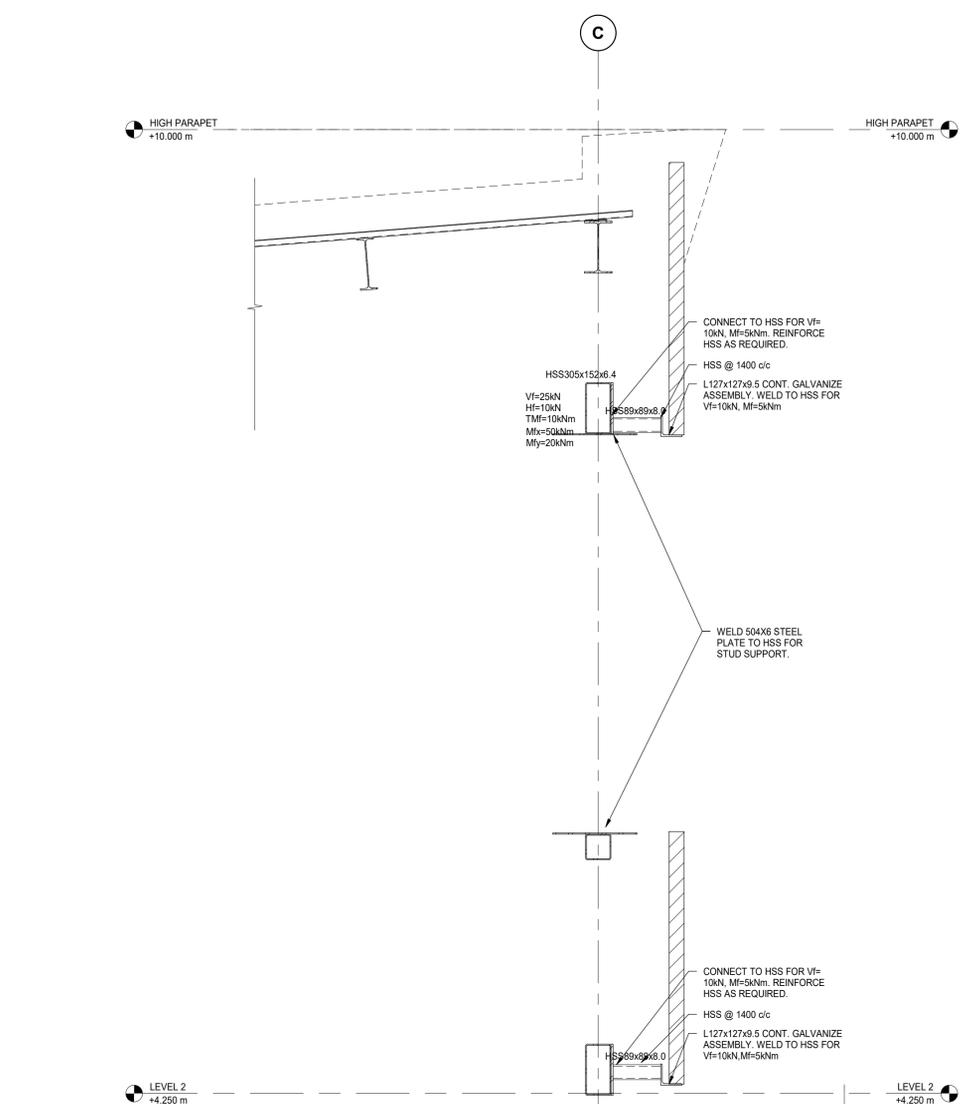
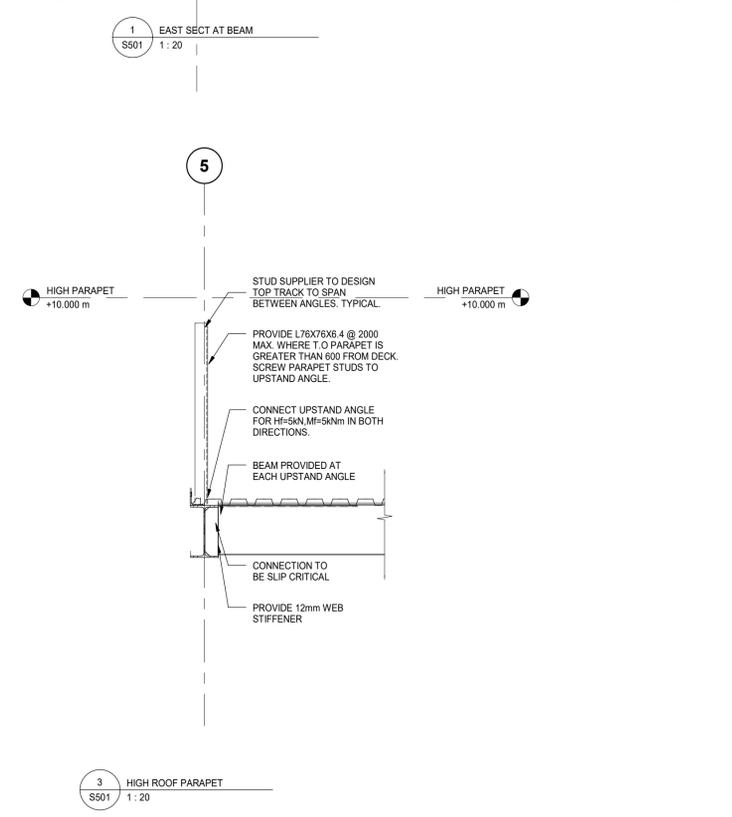
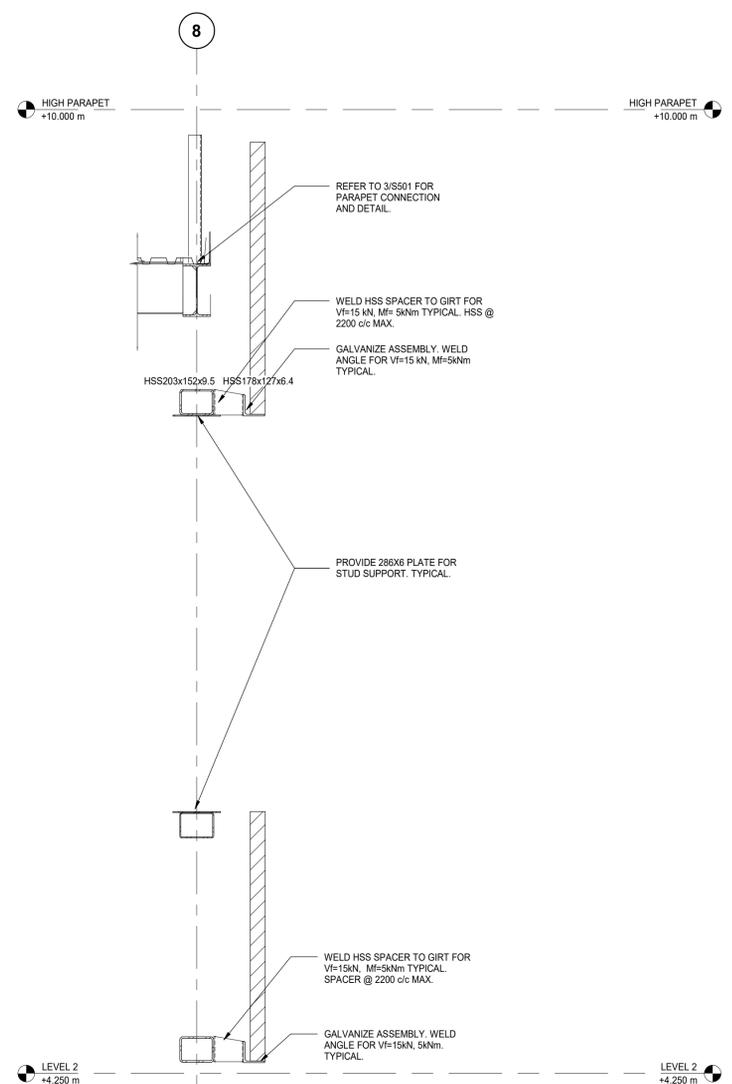
SEAL:

DRAWN: <b>DM</b>	CHECKED: <b>IFM</b>
SCALE: <b>1 : 20</b>	PROJECT NUMBER: <b>210112</b>

SHEET TITLE:  
**DETAILED SECTIONS**

**S500**





Contractor must check and verify all dimensions on the job, and report any discrepancies to the Architect before proceeding with the work.

Do not scale this drawing.

ISSUE:  
ISSUED FOR CONSTRUCTION

MARK	DATE	DESCRIPTION
10	2022/07/15	ISSUED FOR CONSTRUCTION
9	2022/05/30	ISSUED FOR STEEL RESTRUCTURING
8	2022/01/31	REVISED ISSUED FOR TENDER/ISSUED FOR BUILDING PERMIT
7	2021/09/28	ADDENDUM S2
6	2021/09/14	ADDENDUM S1
5	2021/09/09	ISSUED FOR BUILDING PERMIT
4	2021/08/30	ISSUED FOR TENDER
3	2021/08/25	ISSUED FOR TENDER REVIEW

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PROJECT NAME:  
**NEW SAYERS FOOD STORE BURLEIGH STREET, APSLEY**

PROJECT ADDRESS:  
**132 Burleigh Street**

SEAL:

DRAWN:  
DM

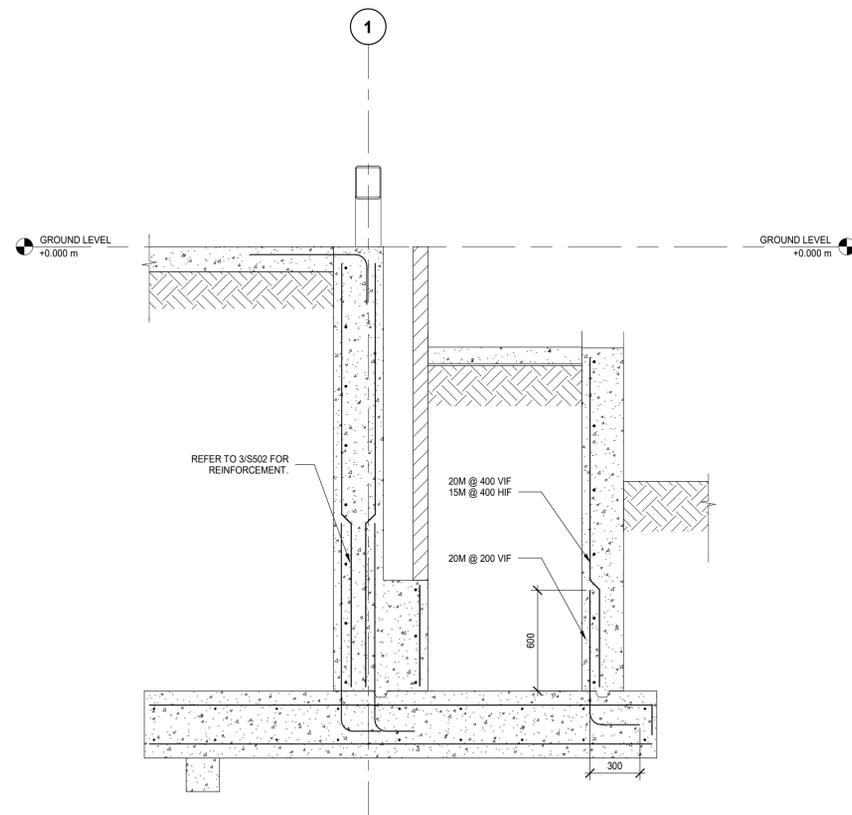
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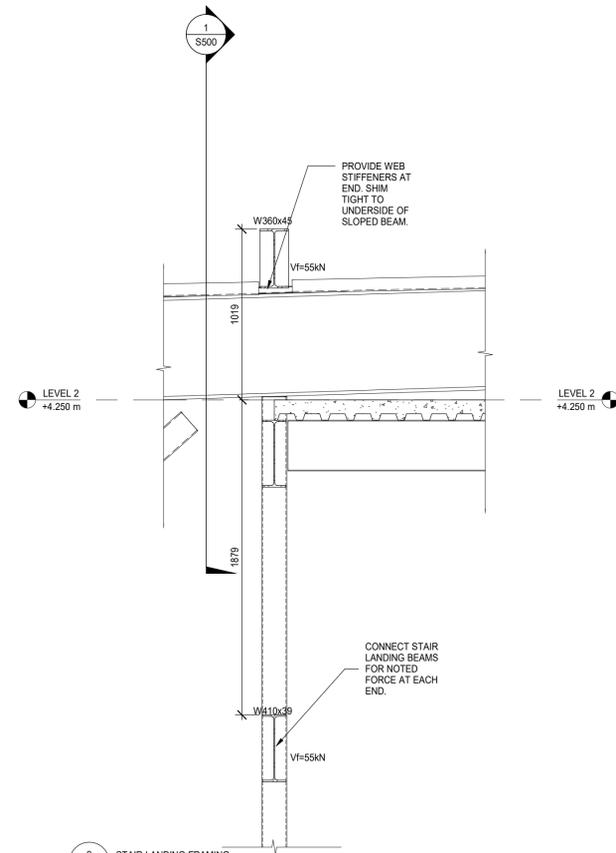
PROJECT NUMBER:  
210112

SHEET TITLE:  
**DETAILED SECTIONS**

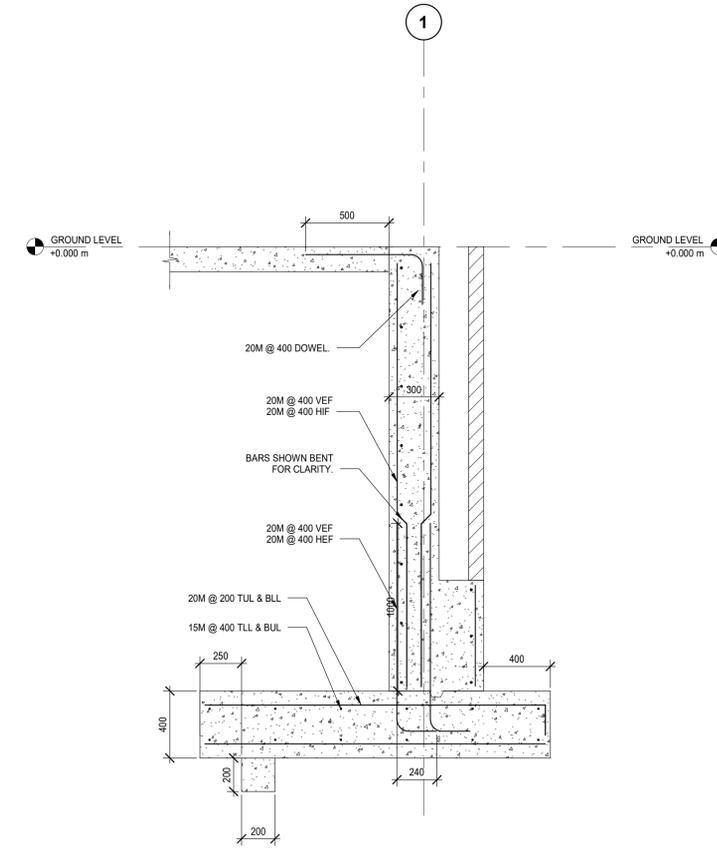
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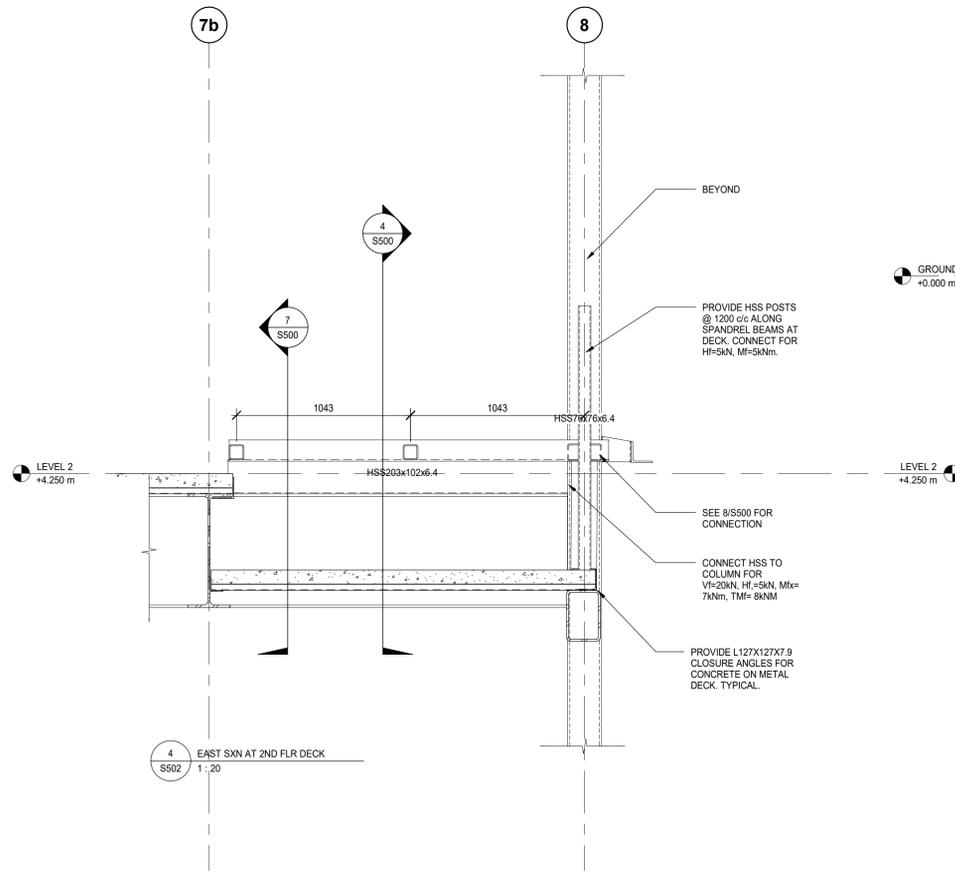
1 RETAINING WALLS AT RAMP  
S502 1:20



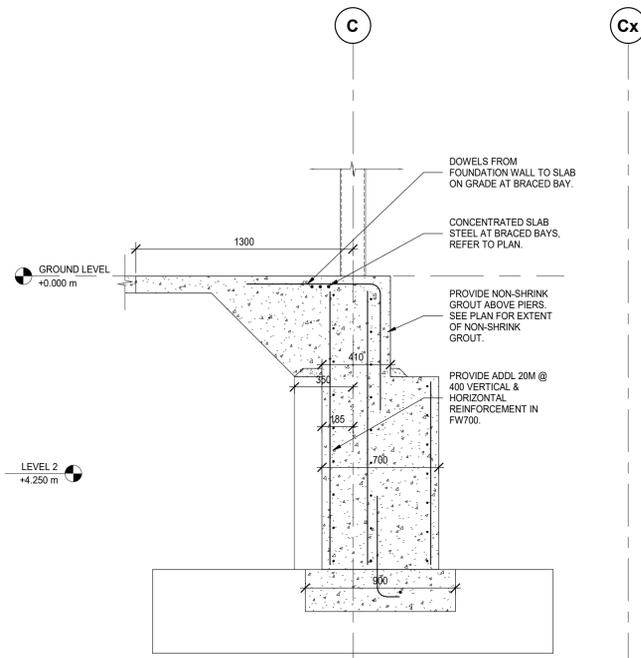
2 STAIR LANDING FRAMING  
S502 1:20



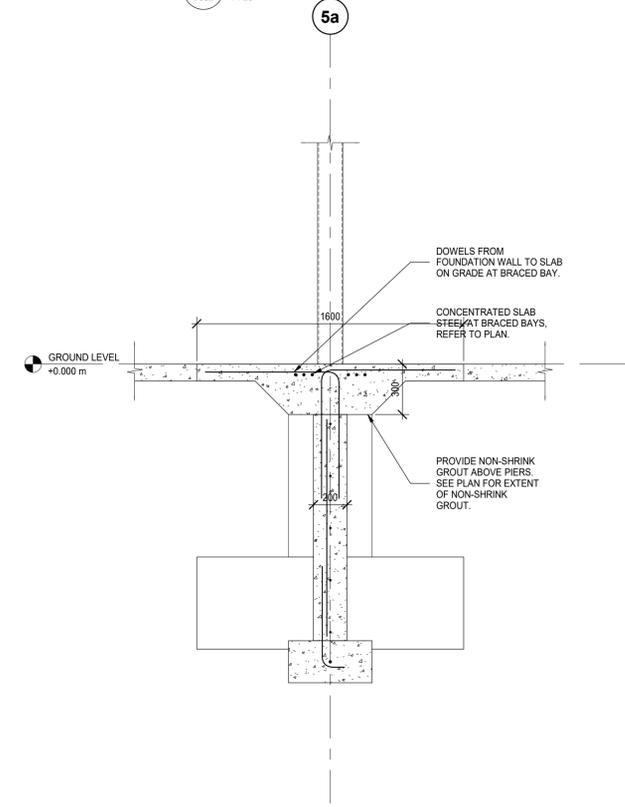
3 RETAINING WALL  
S502 1:20



4 EAST SXN AT 2ND FLR DECK  
S502 1:20



5 G/L BRACED BAY WALL SECT  
S502 1:20



6 INTERIOR BRACED BAY WALL SECT.  
S502 1:20

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SEAL:

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SCALE: 1 : 20	PROJECT NUMBER: 210112

SHEET TITLE:  
**DETAILED SECTIONS**

**S502**

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SCALE: As indicated	PROJECT NUMBER: 210112

SHEET TITLE:  
**DETAILED SECTIONS**

**S503**

