

4.01STEEL DESIGN TO CSA S16.

4.02 WELDED CONNECTIONS TO CSA W59.

4.03BOLTS FOR STEEL CONNECTIONS TO ASTM A325 U.N.O.

4.04 BOLTS FOR ALL OTHER CONNECTIONS TO ASTM A307M U.N.O.

4.05 ROLLED OR WELDED STRUCTURAL QUALITY STEEL TO CSA G40.20/G40.21 350W.

4.06 HOLLOW STRUCTURAL SECTIONS TO CSA G40.20/G40.21 350W U.N.O. 4.07 ALL STEEL SHALL BE SUPPLIED WITH SHOP PRIME FINISH AND SHALL

BE PAINTED U.N.O.

4.08 SHOP DRAWINGS SHALL BE PROVIDED TO THE BUILDING ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION. SHOP DRAWINGS SHALL BE SEALED BY A QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. SHOP DRAWINGS SHALL BE COORDINATED WITH OTHER TRADES.

#### 5.0 PRE-ENGINEERED STRUCTURAL STEEL

5.01SHALL BE DESIGNED TO CSA S16, CSA S136, AND THE ONTARIO BUILDING CODE.

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#### 6.0 MISCELLANEOUS STEEL

6.01MISCELLANEOUS STEEL SHALL INCLUDE STAIRS, HANDRAILS, GUARDRAILS, STEEL STUD WALLS AND SUPPORT STRUCTURES INDICATED, BUT NOT OTHERWISE DETAILED ON THE CONTRACT DRAWINGS. MISCELLANEOUS STEEL SHALL BE DESIGNED TO CSA S16, CSA S136, AND THE ONTARIO BUILDING CODE.

6.02 SHOP DRAWINGS SHALL BE PROVIDED TO THE BUILDING ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION. SHOP DRAWINGS SHALL BE SEALED BY A QUALIFIED PROFESSIONAL ENGINEER LICENSED IN THE PROVINCE OF ONTARIO. SHOP DRAWINGS SHALL BE COORDINATED WITH OTHER TRADES.

#### 7.0 MASONRY

7.01MASONRY DESIGN TO CSA S304.1

7.02 CONCRETE MASONRY UNITS (METRIC) TO CSA A165.1 TO A165.3 U.N.O. EXPOSED CORNERS SHALL BE SUPPLIED WITH RADII.

7.04 MASONRY VENEER SHALL BE AS INDICATED AND AS SELECTED BY

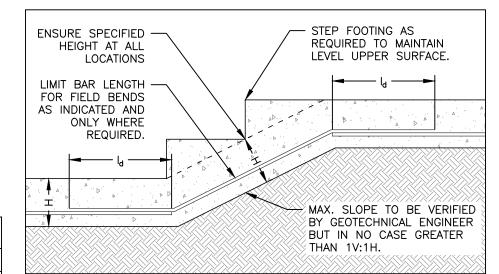
7.05 VENEER ANCHORS SHALL BE SPACED AT 600H BY 400V MAX. OR 400H BY 600V MAX. AS APPLICABLE. VENEER ANCHORS SHALL BE STAINLESS STEEL BY BLOK-LOK OR APPROVED EQUAL. ANCHORS SHALL BE SELECTED BASED ON INDICATED BACKING STRUCTURE SIZED FOR FULL THICKNESS OF INSULATION, AIR-SPACE, ETC.

7.06 SUBMITTALS SHALL BE PROVIDED TO THE BUILDING ENGINEER FOR REVIEW PRIOR TO CONSTRUCTION.

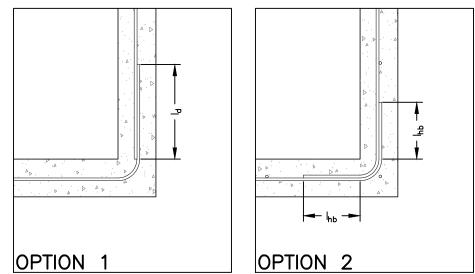
SAW CUTTING OF SLABS ON GRADE TYP. CONTRACTION JOINTS SHALL BE SAW CUT TO 1:4 THE THICKNESS

OF THE SLAB. CONTRACTION JOINTS SHALL BE LOCATED ALONG INTERIOR GRID LINES AND AT REGULAR INTERVALS BUT NOT MORE THAN 25 TIMES THE SLAB THICKNESS. SAW CUTS SHALL BE FULLY CLEANED AND FILLED WITH BACKER ROD AND SEALANT. JOINTS SHALL EXTEND DIAGONALLY AROUND COLUMNS, DRAINS AND SIMILAR OBSTRUCTIONS.

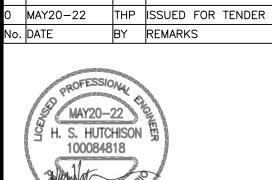
## CHANGES IN FOOTING ELEVATION TYP.



#### REINFORCING LAPS AT WALL CORNERS TYP.



- THE CONTRACTOR SHALL ENSURE THAT ALL WORK IS IN CONFORMANCE WITH THE ONTARIO BUILDING CODE, THE OCCUPATIONAL HEALTH AND SAFETY ACT AND ALL OTHER GOVERNING REGULATIONS IN FORCE AT THE TIME OF
- THE CONTRACTOR SHALL CONFIRM ALL DIMENSIONS AND E RESPONSIBLE FOR SAME. THE CONTRACTOR SHALL NOTIFY ALL REQUIRED PARTIES OF ANY INCONSISTENCIES PRIOR COMMENCING THE WORK.
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- THE CONTRACTOR SHALL HAVE THE SOLE RESPONSIBILITY FOR THE DESIGN, ERECTION, OPERATION, MAINTENANCE AND REMOVAL OF TEMPORARY BRACING, SHORING, HOARDING, AND ANY FACILITIES OR METHODS REQUIRED TO KEEP TH CONSTRUCTION SAFE, PLUMB, LEVEL AND IN TRUE ALIGNMENT AT ALL PHASES OF THE WORK UNTIL COMPLETION.





NEW FIRE STATION No. HIGHLANDS BOULEVARD MILLBROOK, ON

PRE-ENG REQUIREMENTS GENERAL NOTES AND STANDARD DETAILS





**S100** 

MAY19-

164.21.005

# XPS -EXTRUDED POLYSTYRENE INSULATION GENERAL DESIGN ALL DESIGN WORK IN CONFORMANCE WITH THE ONTARIO BUILDING

- SITE SPECIFIC DESIGN DATA NEAR CAVAN ONTARIO (CLOSEST TABULATED LOCATION TO MILLBROOK) TAKEN FROM MMAH SUPPLEMENTARY STANDARD SB-1, TABLE 2 AND 3, EFFECTIVE DATE: JANUARY 1, 2020.
- CLIMATE ZONE 6.

MBG -METRES BELOW GRADE

-NOT APPLICABLE

-REFER TO (REFERENCE)

SPMDD -STANDARD PROCTOR MAXIMUM DRY DENSITY

-NOT REQUIRED

-NOT SHOWN

-ON CENTRE

-NUMBER

-PLATE

-SIMILAR

TBC -TO BE CONFIRMED

TBD -TO BE DETERMINED

-TOP OF

-TYPICAL

-UNDERSIDE

-VERTICAL

-WITH

-SLAB ON GRADE

U.N.O. -UNLESS NOTED OTHERWISE

WWR -WELDED WIRE REINFORCING

REQ'D -REQUIRED

-MINIMUM

-MINUTES

min

N/A

N/S

OC

SIM.

SOG

T/0

TYP.

U/S

- LIVE LOADS DUE TO USE AND OCCUPANCY: 12.0 kPa GVWR >9,000kg GARAGE FLOOR AREAS ROOF AREAS 1.00 kPa OTHER FLOOR AREAS 4.80 kPa
- CONCENTRATED LIVE LOADS FLOORS ROOF SURFACES
- OVER 750X750 9.00 kN 1.30kN OVER 200X200

- $p = I_w q C_e C_t C_p C_g$  $p_{i} = l_{w}qC_{ei}C_{t}C_{pi}C_{gi}$
- SEISMIC DATA (CAVAN): 0.140  $S_0(0.2)$  $S_a(0.5)$ 0.092 0.055  $S_a(1.0)$ 0.0280  $S_a(2.0)$ 0.0071  $S_{a}(5.0)$ S<sub>a</sub>(10) 0.0030 PGA 0.086 PGV 0.074 SITE CLASS
- $I_EF_sS_a(0.2)$ 0.21>0.16 0.042>0.03  $I_EF_sS_a(2.0)$ 0.8PGA=0.0688 PGAREF  $F_a = 1.0$ Fv=1.0
- $I_E F_a S_a(0.2) = 0.21 < 0.35$ SFRS - MASONRY STRUCTURES  $R_d=1.5$   $R_o=1.5$   $h_n=3.3$   $T_a=0.122$

 $V=S(T_a)M_vI_EW/(R_dR_o)=0.088W$ 

 $V=S(T_a)M_vI_EW/(R_dR_o)=0.102W$ SFRS - STEEL STRUCTURES (FOUNDATIONS ONLY TBC BY SUPPLIER)  $R_d=1.5 R_o=1.3 h_n=6.8 T_a=0.358$ 

#### **FOUNDATION**

- FOUNDATION DESIGN IN CONFORMANCE WITH GEOTECHNICAL INVESTIGATION PROPOSED NEW FIRE HALL BUILDING - 988 COUNTY ROAD 10, MILLBROOK, ONTARIO, DATED MARCH 17, 2022, PREPARED BY GHD LIMITED (REFERENCE: 11231078) FOR THE TOWNSHIP OF CAVAN MONAGHAN AND ADDENDUM #1, DATED 8 APRIL 2022.
- FOUNDATION DESIGN BASED ON A GEOTECHNICAL REACTION AT SERVICEABILITY LIMIT STATE (SLS) OF 200kPa AND A FACTORED ( $\phi$ =0.5) GEOTECHNICAL RESISTANCE AT THE ULTIMATE LIMIT STATE OF

FLOOR SYSTEM (MEZZANII	NE)
2" CONCRETE TOPPING	1.2 kPa
8" PRE-CAST HCS	2.6 kPa
SUSPENDED CEILING	0.10 kPa
M&E FIXTURES & EQUIPMENT	0.25 kPa
	4.15 kPa

CANOPY		
ROOFING	0.10	kPa
GYPSUM SHEATHING	0.10	kPa
1½" STEEL DECKING	0.15	kPa
STEEL STRUCTURE	0.10	kPa
1½" STEEL SOFFIT	0.15	kPa
M&E FIXTURES	0.15	kPa
	0.75	kPa

### MATERIAL DESIGN AND CONSTRUCTION

- 1.0 CONCRETE
- 1.01CONCRETE DESIGN TO CSA A23.3.
- 1.02 CONCRETE MATERIALS AND METHODS OF CONCRETE CONSTRUCTION TO CSA A 23.1.
- 1.03 TEST METHODS AND STANDARD PRACTICES FOR CONCRETE TO CSA

1.04 CONCRETE MATERIALS SHALL BE PROVIDED BY AN RMCAO CERTIFIED

- 1.05 CONCRETE MATERIAL SUBMISSIONS SHALL BE PROVIDED BY THE CONTRACTOR PRIOR TO CONSTRUCTION. CONCRETE SUBMITTALS SHALL BE IN CONFORMANCE WITH RMCAO STANDARD FORMS.
- 1.06 CONTINUOUS 90mm WIDE BY 40mm DEEP KEYS SHALL BE FORMED INTO CONCRETE AT JOINTS BETWEEN FOOTINGS AND WALLS AND AT ALL CONTRUCTION JOINTS WHERE REQUIRED.
- 1.07 SLABS, SLABS-ON-GRADE AND EXTERIOR FLATWORKS SHALL BE SEPARATED FROM WALLS, CURBS, PIERS AND SIMILAR VERTICAL SURFACES WITH ASPHALT IMPREGNATED FIBERBOARD OR SIMILAR MATERIALS U.N.O. EXPOSED JOINTS SHALL BE SEALED WITH BITUMINOUS SEALANT U.N.O.

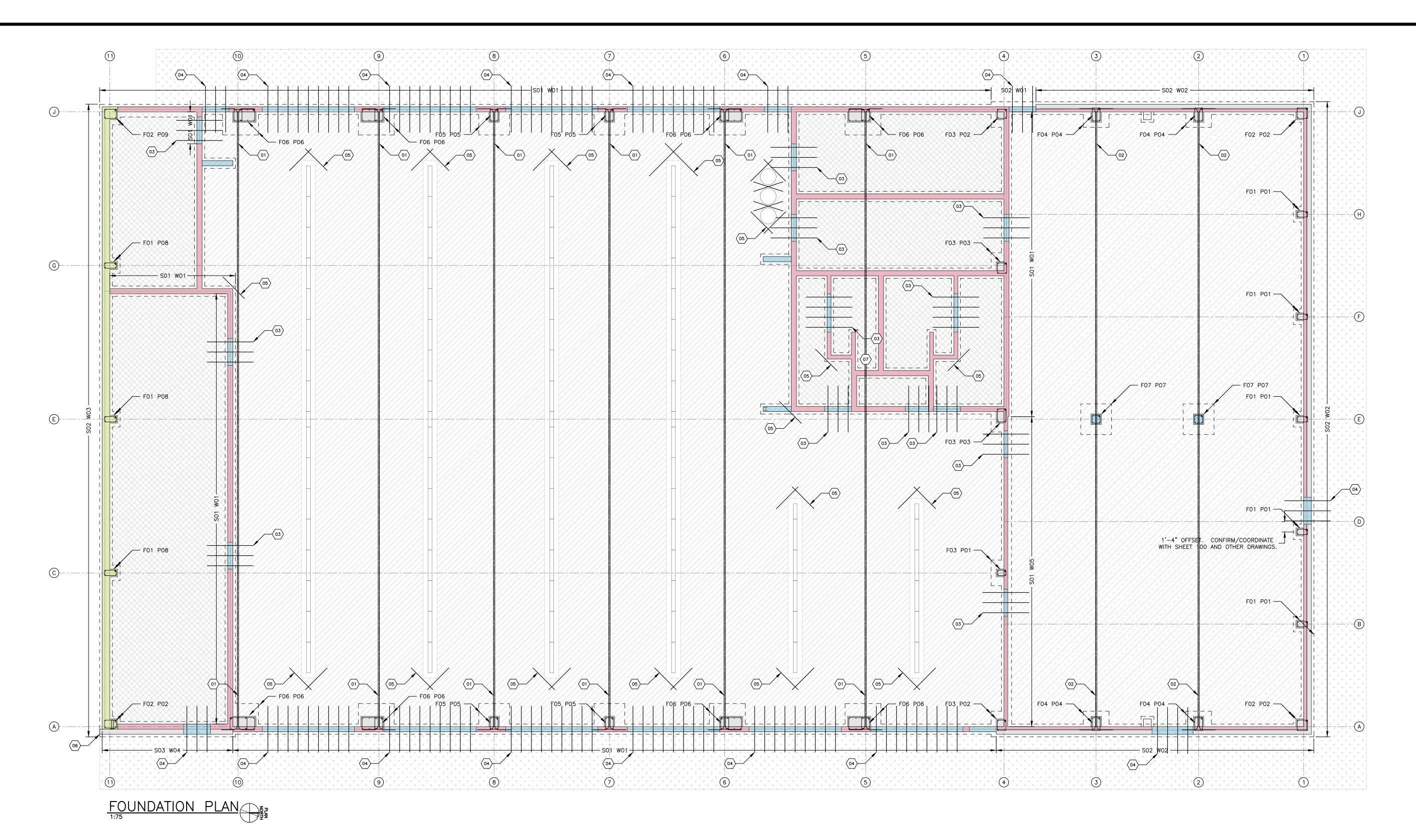
#### 2.0 PRE-CAST CONCRETE

- 2.01DESIGN TO CSA A23.3
- 2.02 MATERIALS AND CONSTRUCTION TO CSA A23.4
- 2.03U.N.O. PRE-CAST SHALL BE DESIGNED FOR LATERAL LOADS DUE TO SEISMIC ACTION

MIN. LA	MIN. LAP LENGTHS (Id) FOR REBAR									
	20MPa	25MPa	30MPa	32MPa	35MPa					
10M	325	290	265	255	245					
15M	485	435	395	385	370					
20M	645	580	530	510	490					
25M	1010	900	825	800	765					
30M	1210	1080	990	955	915					
35M	1410	1260	1155	1115	1065					

### 90° TENSION HOOKS (Inb) FOR REBAR

	20MPa	25MPa	30MPa	32MPa	35MPa
10M	224	200	183	177	169
15M	335	300	274	265	254
20M	447	400	365	354	338
25M	559	500	456	442	423
30M	671	600	548	530	507
35M	783	700	639	619	592



### LEGEND SHEET S101

T/O CONCRETE AT FFE+0 T/O CONCRETE AT FFE-9"

FOUNDATION WALL/CURB T/O CONCRETE AT FFE+8". CONFIRM/COORDINATE WITH MASON. CURB WIDTH MAY BE REDUCED TO MATCH MASONRY WIDTH WHERE APPLICABLE.

FOUNDATION WALL T/O CONCRETE AT FFE+118½". CONFIRM/COORDINATE FINISHED HEIGHT WITH HCS SUPPLIER TO ENSURE PROPER PLACMENT OF BEARING PADS ETC.

200 (8") INTERIOR SOG. FL01 CONCRETE. REINFORCE WITH 15M@500 E/W. CONTRACTION JOINTS INSTALLED AT NOT MORE THAN 5000 (16'-8") O.C.

165 (6½") INTERIOR SOG. FL01 CONCRETE. REINFORCE WITH 10M@300 E/W. CONTRACTION JOINTS INSTALLED AT NOT MORE THAN 4100 (13'-4") O.C. THICKEN SLAB TO 200 (8") WHERE TIE REBAR REQUIRED.

150 (6") INTERIOR SOG. FLO1 CONCRETE. REINFORCE WITH 10M@300 E/W. CONTRACTION JOINTS INTALLED AT NOT MORE THAN 3750 (12'-2") O.C. THICKEN SLAB TO 200 (8") WHERE TIE REBAR REQUIRED. 1 200 (8") EXTERIOR SOG. FLO2 CONCRETE. TOOLED CONTRACTION JOINTS AT NOT MORE THAN 3000 O.C. FORMED EXPANSION JOINTS CENTRED BETWEEN GARAGE DOORS AND AT NOT MORE THAN 6000 O.C.

#### SCHEDULE S-01 - CONCRETE SPECIFICATIONS

I.D.	I.D. MAIN USE	EXPOSURE 1	MAX. WATER TO CEMENTING	MINIMUM COMPRESSIVE STRENGTH				AIR	CURING CSA	ACCRECATE	CLASS/FLOOR FINISH ADMIX	
1.0.	MAIN USE		MATERIALS RATIO	3d	7d	28d	56d	CONTENT	CURING CSA A23.1 TABLE 20	AGGREGATE	AGGREGATE	GATE CLASS/FLOOR FINISH CSA A23.1 TABLE 22
FT01	FOOTINGS, INTERIOR PIERS	N	0.65	8MPa	N/A	20MPa	N/A	0	1 (3d)	20mm	N/A	
FL01	GROUND FLOOR SLAB ON GRADE	C-4	0.55	10MPa	N/A	25MPa	N/A	0	1 (3d)	20mm	B/STEEL TROWEL	
FL02	EXTERIOR FLATWORK	C-2	0.45	N/A	22.4MPa	32MPa	N/A	5% TO 8%	2 (7d)	20mm	вкоом	
FL03	TOPPING SLAB	N	0.55	10MPa	17.5MPa	25MPa	N/A	0	1 (3d)	10mm	B/STEEL TROWEL	
FW01	FOUNDATION WALLS	F-2	0.55	10MPa	N/A	25MPa	N/A	4% TO 7%	1 (3d)	20mm	N/A	
SD01	GENERAL USE	N	0.65	8MPa	N/A	20MPa	N/A	0	1 (3d)	20mm	N/A	
RW01	RETAINING WALL	C-1	0.40	14MPa	24.5MPa	35MPa	N/A	5% TO 8%	2 (7d)	20mm	N/A	

## KEY NOTES SHEET S101

- $\langle 01 \rangle$  2-30M REBAR TENSION STEEL CONT. BETWEEN OPPOSING PIERS. SIZE TO BE CONFIRMED DURING PRE-ENG BUILDING SHOP DRAWING REVIEW. DO NOT FIELD BEND BARS. PROVIDE COUPLERS, BAR-LOCK "L-SERIES" COUPLERS BY DAYTON SUPERIOR CANADA LTD. OR APPROVED EQUAL, WHERE REQUIRED TO FACILITATE OTHER WORK.
- 2-20M REBAR TENSION STEEL CONT. BETWEEN OPPOSING PIERS. SIZE TO BE CONFIRMED DURING PRE-ENG BUILDING SHOP DRAWING REVIEW. DO NOT FIELD BEND BARS. PROVIDE COUPLERS, BAR-LOCK "L-SERIES" COUPLERS BY DAYTON SUPERIOR CANADA LTD. OR APPROVED EQUAL, WHERE REQUIRED TO FACILITATE OTHER
- 03 1800 (6'-0") LONG 20M@400 REBAR AT OPENING AND/OR AS INDICATED. BEND REBAR WHERE REQUIRED TO FACILITATE SLOPED SOG.
- 1800 (6'-0") LONG 20M@400 STAINLESS STEEL REBAR AT ALL EXTERIOR OPENINGS. BEND REBAR WHERE REQUIRED TO FACILITATE SLOPED SOG.  $\langle 05 \rangle$  1200 (4'-0") LONG 20M CENTRED AT CORNERS (TRENCH DRAINS, BETWEEN HATCHES, AND AS INDICATED).
- OFFSET TIE REBAR TO FACILITATE MECHANICAL OPENINGS WHERE REQUIRED. REFER TO MECH DRAWINGS. OFFSETS SHALL BE REVIEWED BY ENGINEER PRIOR TO CONCRETE PLACEMENT.

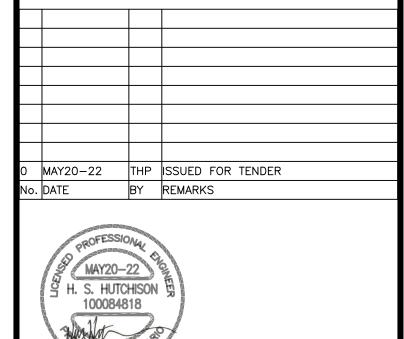
 $\langle ^{06} \rangle$  extend 8" foundation wall 4" beyond corner for masonry veneer support.

SCHEDULE S-02 - FOUNDATION FOOTINGS

I.D.	LOCATION	WIDTH	LENGTH	MIN. HEIGHT	TOP OF FOOTING	WIDTH REBAR	LENGTH REBAR	CONCRETE	NOTES	3
S01	STRIP FOOTING	16"	CONT.	10"	FFE-50"	N/A	2-20M CONT.	FT01		
502	STRIP FOOTING	20"	CONT.	10"	FFE-50"	N/A	3-20M CONT.	FT01		13 Cc
503	STRIP FOOTING	24"	CONT.	10"	FFE-50"	N/A	3-20M CONT.	FT01		Banc
504	STRIP FOOTING	14"	CONT.	10"	FFE-50"	N/A	2-20M CONT.	FT01		613.3 greer
01	EXTERIOR FOOTING	32"	24"	10"	FFE-50"	3-20M 26" LONG	2-20M 18" LONG*	FT01		DESIG
02	CORNER FOOTING	32"	32"	10"	FFE-50"	2-20M 26" LONG*	2-20M 26" LONG*	FT01		DESIG
03	CORNER FOOTING	40"	32"	10"	FFE-50"	2-20M 34" LONG*	2-20M 26" LONG*	FT01		
04	FRAME END FOOTING	40"	40"	10"	FFE-50"	5-20M 36" LONG	3-20M 34" LONG*	FT01		PROJE 164.2
05	FRAME END FOOTING	48"	48"	10"	FFE-50"	6-20M 42" LONG	4-20M 42" LONG*	FT01		
06	FRAME END FOOTING	48"	56"	10"	FFE-50"	7-20M 42" LONG	4-20M 50" LONG*	FT01		
07	INTERIOR FOOTING	48"	48"	10"	FFE-30"	6-20M 42" LONG	6-20M 42" LONG	FT01		



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NEW FIRE STATION No. 1 HIGHLANDS BOULEVARD MILLBROOK, ON

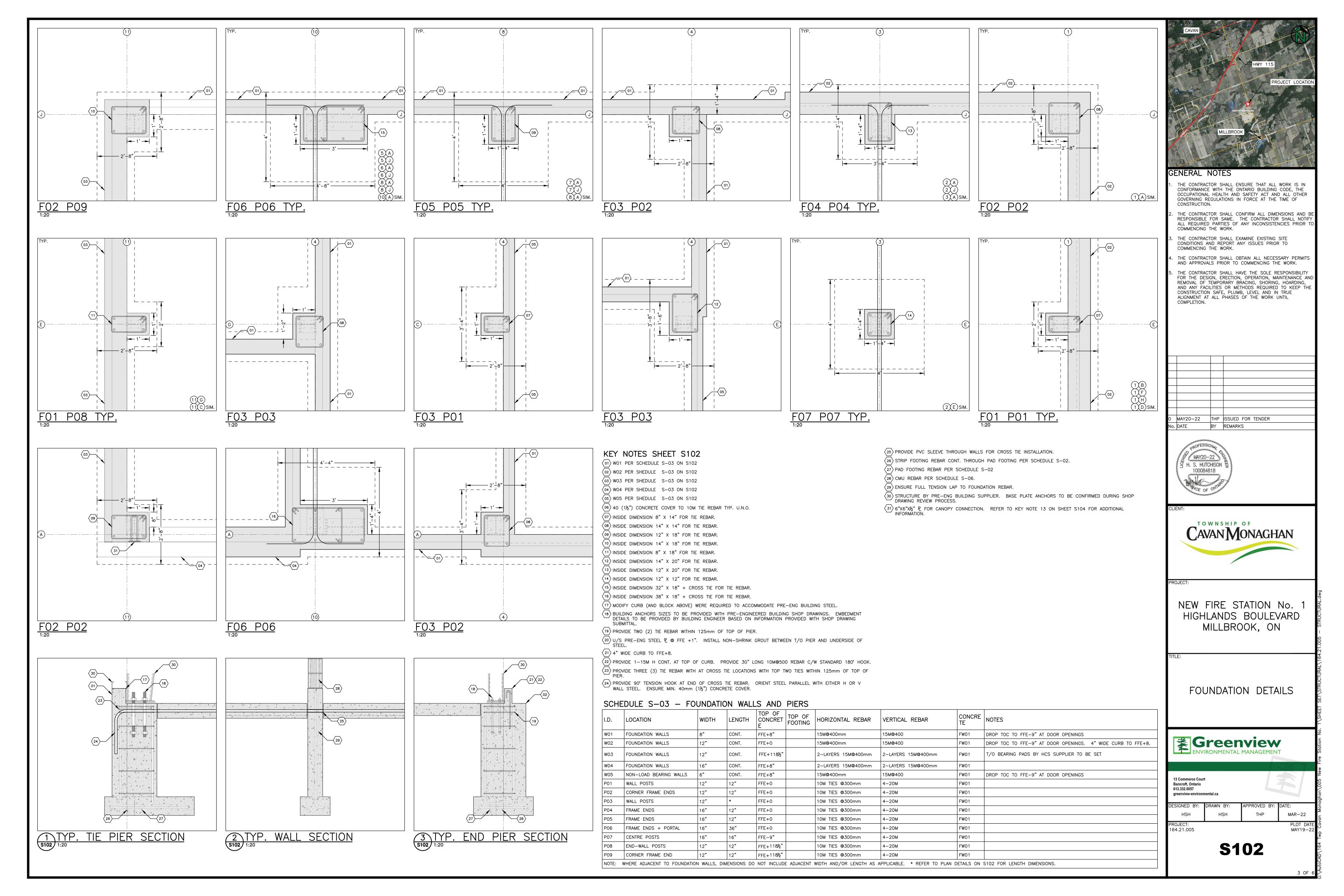
FOUNDATION PLAN

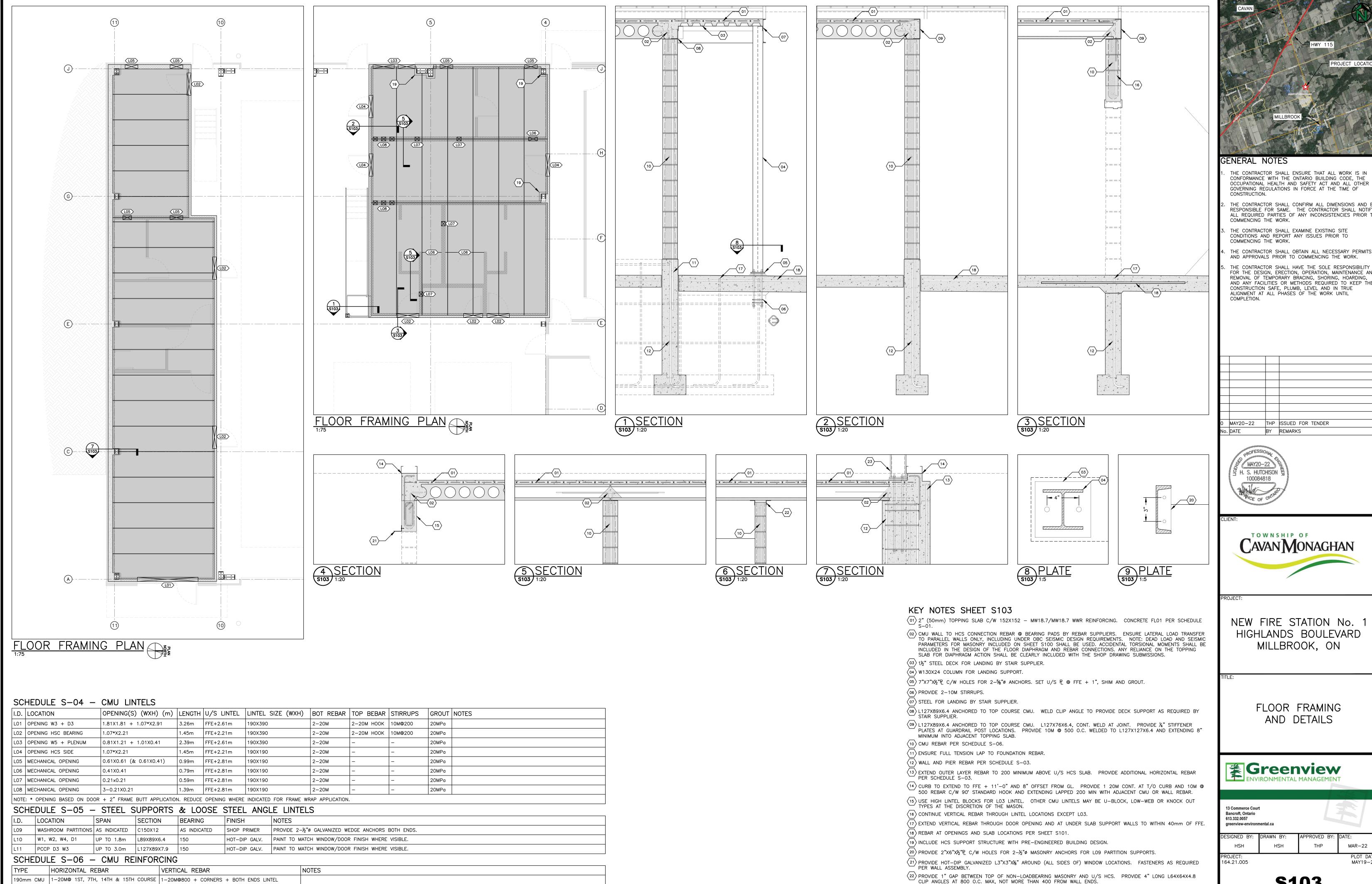


Bancroft, Ontario 613.332.0057 greenview-environmental.c MAR-22

**S101** 

MAY19-2





140mm CMU |1-15M@ 1ST, 7TH, 14TH & 15TH COURSE |1-15M@800 + CORNERS + BOTH ENDS LINTEL

NOTE: FULLY GROUT REBAR CELLS.

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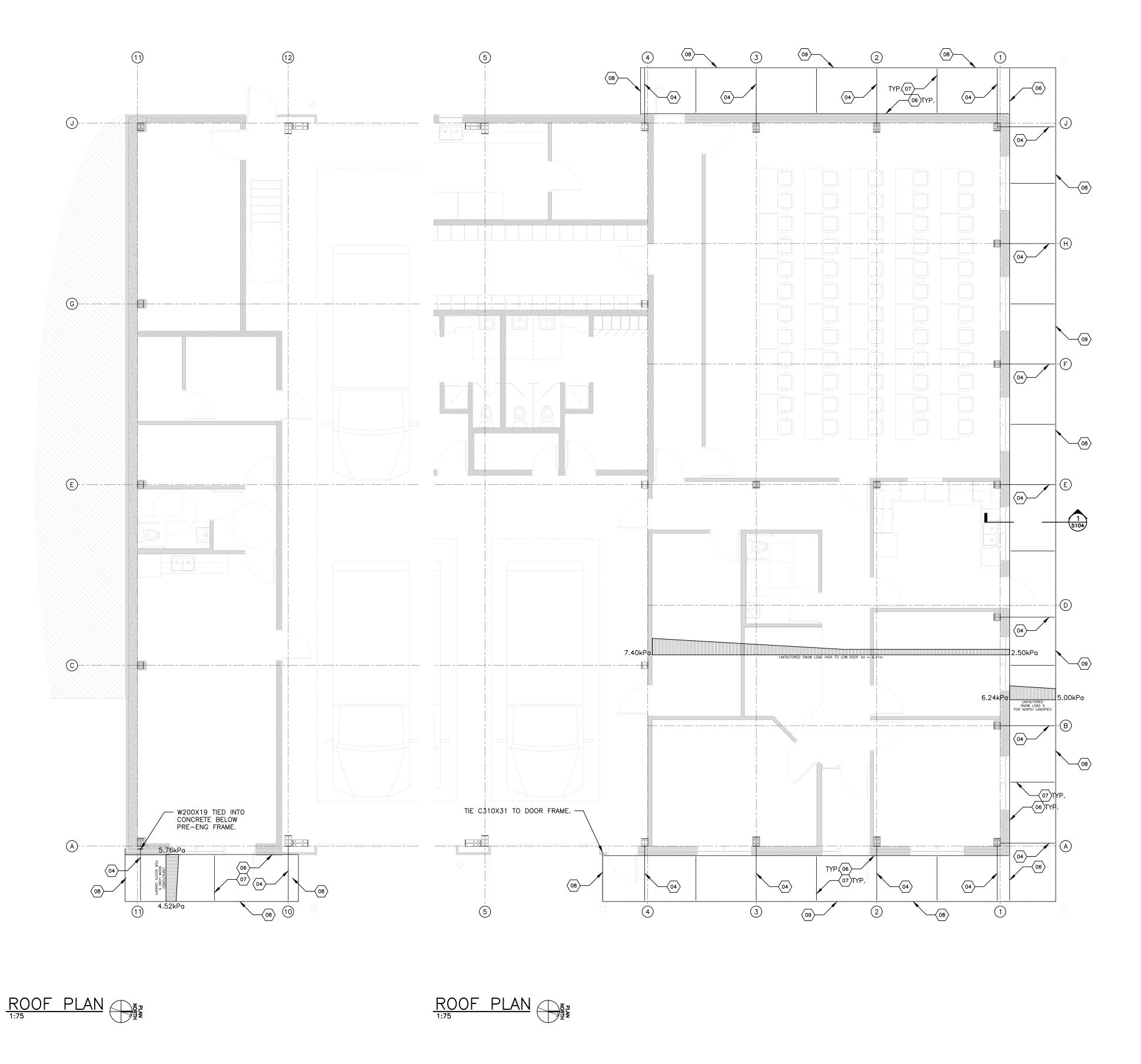
NEW FIRE STATION No. HIGHLANDS BOULEVARD

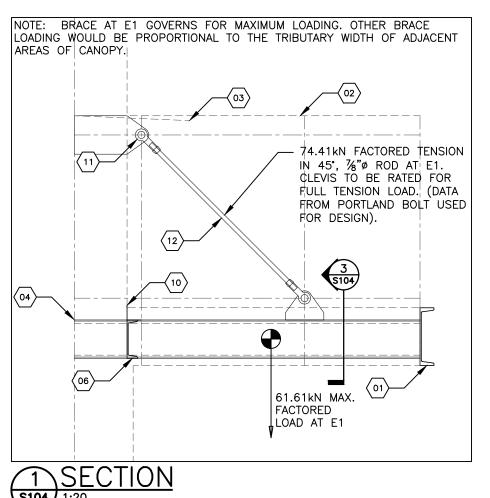
MAR-22

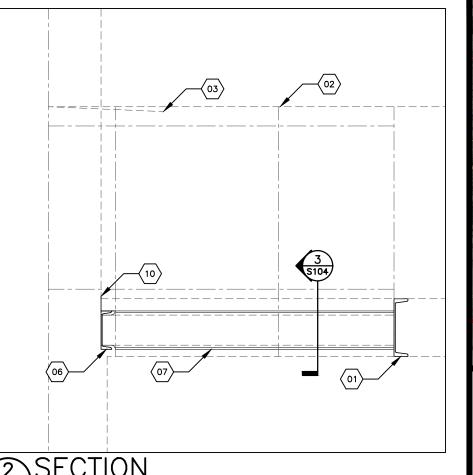
(23) STRUCTURE BY PRE-ENG BUILDING SUPPLIER. BASE PLATE ANCHORS TO BE CONFIRMED DURING SHOP DRAWING

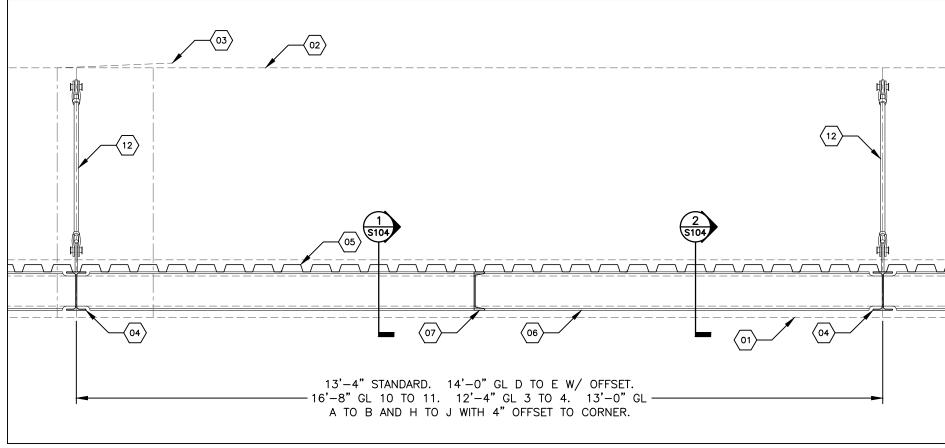
REVIEW PROCESS.

**S103** 









### KEY NOTES SHEET S104

 $\langle 01 \rangle$  U/S STEEL CANOPY AT EL FFE+9'-8". REFER TO OTHER DRAWINGS FOR ADDITIONAL INFORMATION.

02 U/S ROOF PURLINS PER SHEET S100 AND PRE-ENG BUILDING SUPPLIER (FFE+14'-0" MIN.) TO BE CONFIRMED/COORDINATED.

(03) EXTENSION OF ROOF SLOPE BEYOND GL A AND J.

04 W200X19. U/S STEEL AT EL FFE+9'-9½". COORDINATE CONNECTION TO PRE-ENG STEEL.

(05) 1½" STEEL DECK. 18 GAUGE MIN. THICKNESS. MID SPAN DEFLECTION MAY BE LIMITED L/180. 60" NOMINAL LENGTH (SPAN).
(06) C200X17 TYP. SIMPLY SUPPORTED EXCEPT AT OVERHANGS AT A4, J4, A10, AND A11 WHERE MOMENT CONNECTIONS REQUIRED.

(07) C200X17 MID SPAN BRACE. BOLTED CONNECTIONS TO C200X17 AND C310X31.

C310X31 CONT. FULL PERMITER OF CANOPY. PROVIDE WELDED CLIP ANGLES AND BOLTED CONNECTIONS TO END OF W200X19. PROVIDED WELDED CLIP ANGLE FOR BOLTED CONNECTION TO C200X17 BRACE. PROVIDE WELDED CLIP ANGLE SUPPORT FOR OUTER EDGE OF STEEL DECK. ENSURE SUPPORT FROM AT LEAST 2 W200X19 W/O INTERMEDIATE CONSTRUCTION JOINTS. ALL CONNECTIONS SHALL NOT BE VISIBLE FROM OUTER FACE OF CHANNEL.

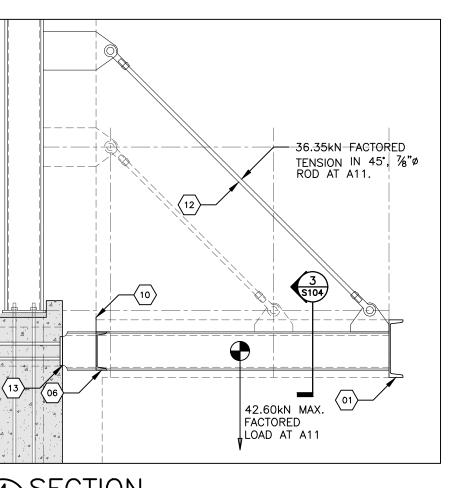
CONSTRUCTION JOINTS FOR C310X31 MAY BE MADE AT INFLECTION POINTS: BETWEEN GL 2 AND GL3 AT THE INFLECTION POINT CLOSEST TO GL2  $(4'-6"\pm \text{ NORTH OF GL 2})$ ; BETWEEN GL B AND D  $(5'-4"\pm \text{ EAST OF D})$ ; AND BETWEEN GL F AND H  $(4'-6"\pm \text{ WEST OF GL F})$ .

CONT. GALVANIZED, COLD—FORM, 18 GAUGE, L3"X3" WELDED BETWEEN TOP OF CHANNEL AND U/S STEEL DECK. ANGLE TO PROVIDE TIE—IN FOR ROOFING SYSTEM AT FACE OF WALL ABOVE. REFER TO BUILDING DRAWINGS FOR ADDITION DETAIL.

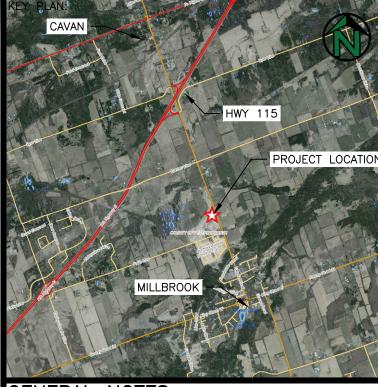
(11) ½" TICK 尼 FOR CLEVIS CONNECTION (1½"Ø PIN TBC). AS CONFIGURED PLATE TO FRAME CONNECTION TO BE PROVIDED FOR FACTORED LOADS OF 52.61kN V, 52.61H AND A MAX. MOMENT OF 18.71kN-m AT GL.

《12》%" ø GALVANIZED STEEL ROD (THREADED ENDS) CONNECTION WITH CLEVIS ENDS. CONFIGURED PORTLAND BOLD CLEVIS NO. 2½ C/W 1½"ø PINS. ALL SUBMISSIONS SHALL BE CONFIRMED FOR APPROPRIATE LOADS.

W200X19. U/S STEEL AT EL FFE+9'-9½". PROVIDE CAST-IN-PLACE 6"X6"X½" 凡 FOR CONNECTION AT A11. 凡 OF 凡 TO BE ALIGNED WITH 凡 OF PRE-ENGINEERED FRAME (ABOVE). PROVIDE 2-15M REBAR BENT AT 180° AND WELDED TO PLATE AT 2 LOCATIONS EACH. CONFIGURE REBAR TO EXTEND TO WITHIN 1½" OF OPPOSITES SIDE OF PIER (14" INCLUDING OUTSIDE RADIUS). CONFIRM COORDINATE WITH REBAR AND PRE-ENG SHOP DRAWING SUBMISSIONS.







#### ENERAL NOTES

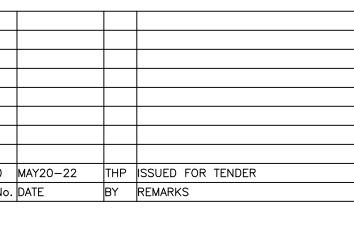
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THE CONTRACTOR SHALL EXAMINE EXISTING SITE CONDITIONS AND REPORT ANY ISSUES PRIOR TO COMMENCING THE WORK.

1. THE CONTRACTOR SHALL OBTAIN ALL NECESSARY PERMITS AND APPROVALS PRIOR TO COMMENCING THE WORK.

5. THE CONTRACTOR SHALL HAVE THE SOLE RESPONSIBILITY FOR THE DESIGN, ERECTION, OPERATION, MAINTENANCE AND REMOVAL OF TEMPORARY BRACING, SHORING, HOARDING, AND ANY FACILITIES OR METHODS REQUIRED TO KEEP THE CONSTRUCTION SAFE, PLUMB, LEVEL AND IN TRUE ALIGNMENT AT ALL PHASES OF THE WORK UNTIL





ENT:



ROJECT:

NEW FIRE STATION No. 1 HIGHLANDS BOULEVARD MILLBROOK, ON

CANOPY



13 Commerce Court
Bancroft, Ontario
613.332.0057
greenview-environmental.ca

DESIGNED BY: DRAWN BY: APPROVED BY: DATE:
HSH HSH THP SEP-21

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