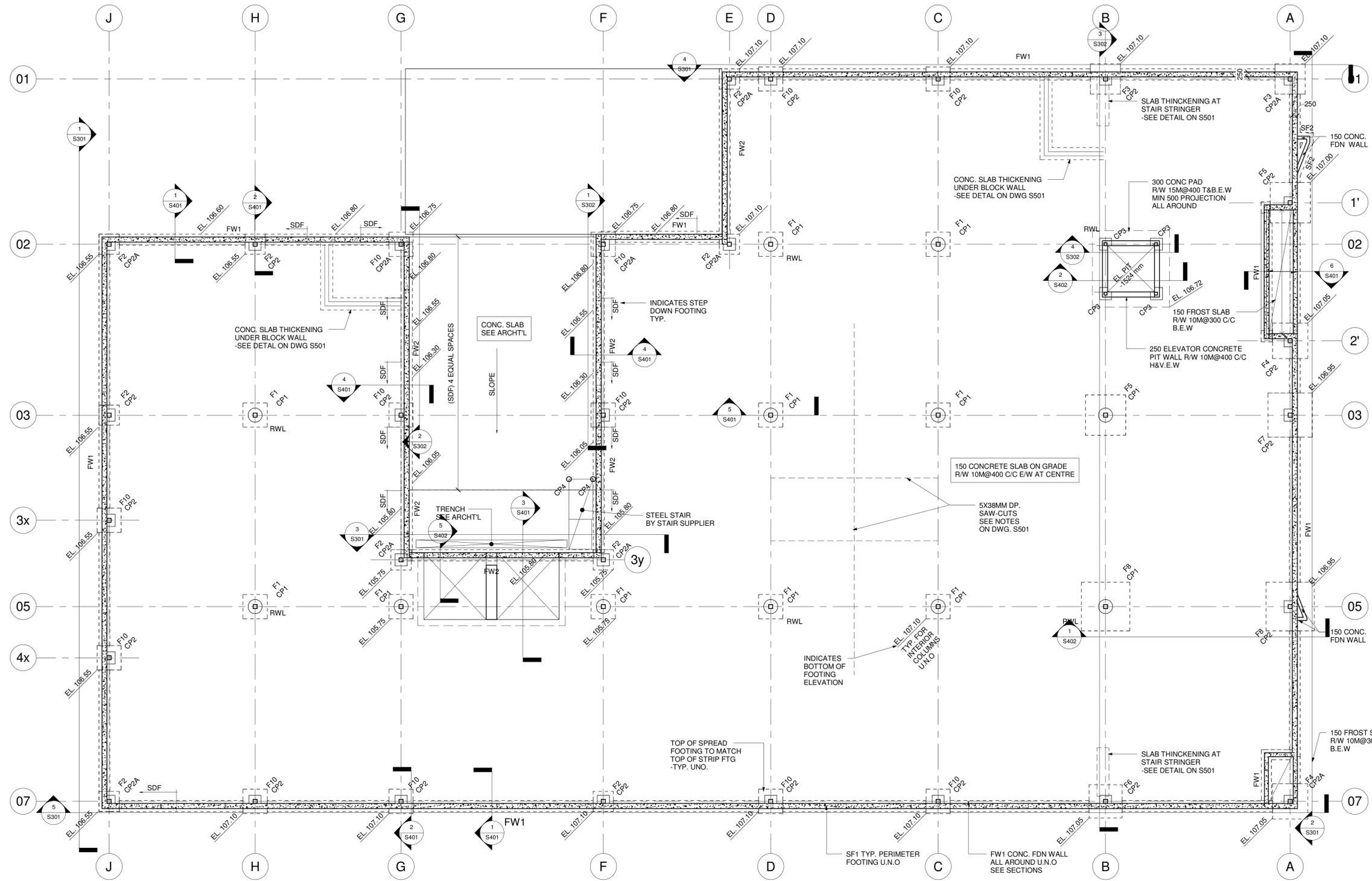


Consultant:

ARCHITECT:

SAPLYS ARCHITECTS INC.
80 ST. CLAIR AVE E. SUITE 806
TORONTO, ONTARIO M4T 1N5
P. 905.510.0595



1 FOUNDATION PLAN

S101 1 : 100

NOTES:

1. TOP OF CONCRETE ELEVATION IS 108.50 (0.0 DATUM) UNLESS CROSSED & NOTED OTHERWISE ON PLAN OR SECTIONS. CONFIRM TOP OF CONCRETE ELEVATION WITH ARCHTL AND CIVIL DRAWINGS PRIOR TO FOOTING CONSTRUCTION.
2. SEE ARCHTL DRAWINGS FOR DIMENSIONS, ELEVATIONS AND SLOPES.
3. PLACE SLAB ON GRADE ON COMPACTED GRANULAR BACKFILL AS RECOMMENDED BY THE SOIL CONSULTANT.
4. CENTER PIERS AND FOOTING UNDER CENTER LINE OF COLUMNS U.N.O. ON PLAN AND/OR SECTIONS.
5. STEP DOWN FOOTING (S.D.F.) AS REQUIRED TO SUIT MECH/ELEC. SERVICES SUCH THAT FOOTING ARE NOT UNDERMINED.
6. ACTUAL FOOTING ELEVATIONS TO BE CONFIRMED BY GEOTECHNICAL ENGINEER FOR THE DESIGN BEARING CAPACITY ON DWG. S501.
7. SEE ALSO GENERAL NOTES AND TYPICAL DETAILS ON DRAWINGS S501 & S502.

No.	Description	Date
2	ISSUED FOR BUILDING PERMIT	JUNE 17, 24
1	ISSUED FOR 50% PROGRESS	MAY 30, 24
Issues / Revision		

CONFIRM ALL GRIDS, DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.
CONTACT ARCHITECT AND ENGINEER FOR ANY DISCREPANCIES.
ONLY LATEST APPROVED DRAWINGS TO BE USED FOR CONSTRUCTION.
ALL DRAWINGS REMAIN THE PROPERTY OF THE CONSULTANT.



AMR ENGINEERING LTD.
STRUCTURAL ENGINEERS
920 Alness Street, Suite 205,
Toronto, ON M3J 2H7
(416) 551 - 1611

Project North



Stamp:



Project : **WAREHOUSE AND OFFICE HEADQUARTERS**

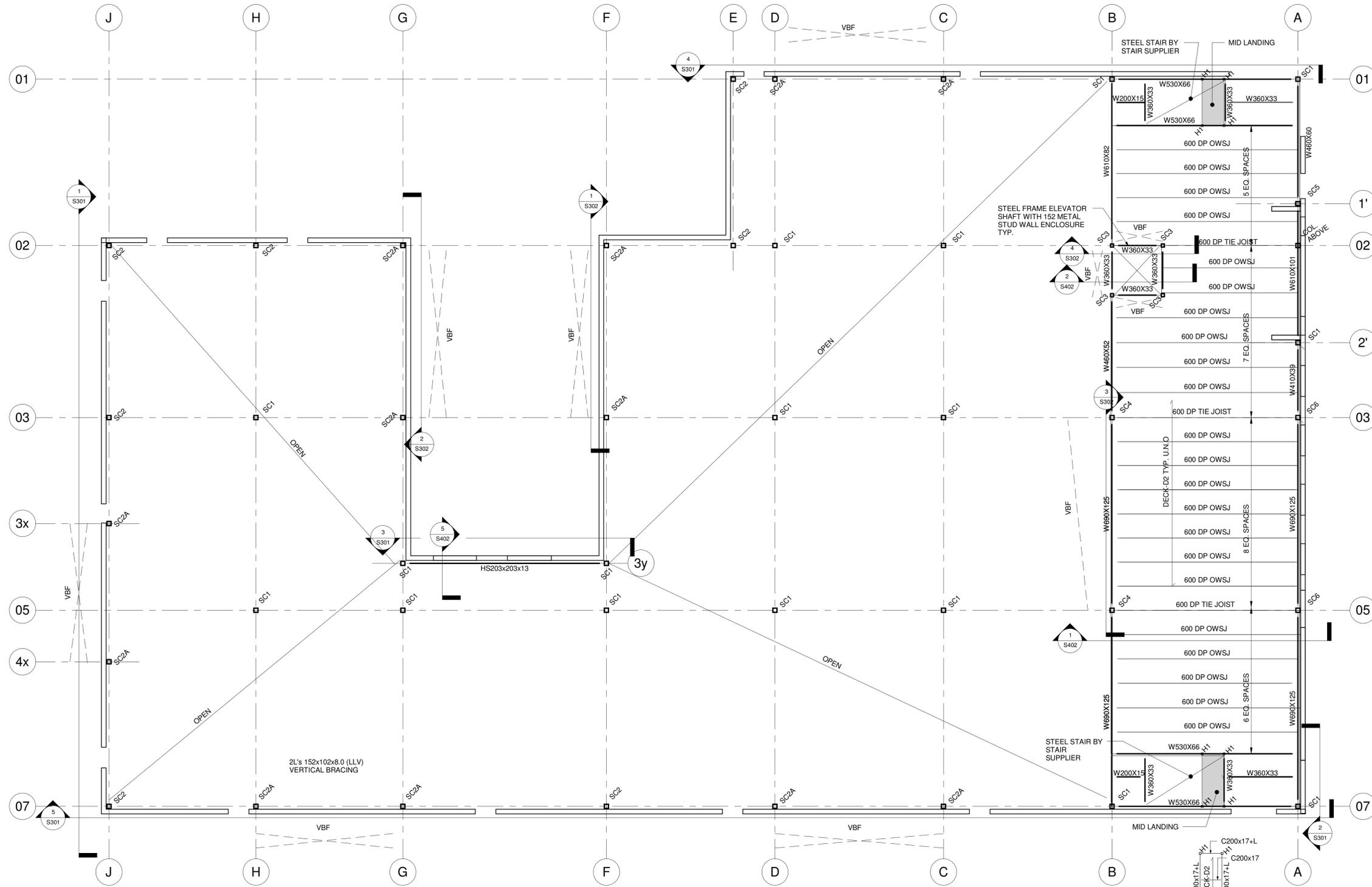
45 BLOWERS CRES
AJAX, ON L1Z 0N4

Drawing Title:
FOUNDATION PLAN

Date: 30-05-2024 Drawing No.

Scale: As Noted **S101**

Drawn By: OH
Checked By: DK Job No: 2280



SECOND FLOOR FRAMING PLAN
1 : 100

NOTES:

- TOP OF CONCRETE AT SECOND FLOOR IS AT ELEVATION +3962mm ABOVE GROUND FLOOR SLAB.
- TOP OF OWSJ'S IS 100MM BELOW TOP OF CONCRETE.
- TOP OF STEEL BEAMS SUPPORTING OWSJ'S IS 200MM BELOW TOP OF CONCRETE UNLESS NOTED OTHERWISE ON PLAN AND/OR SECTIONS. TOP OF STEEL BEAMS SUPPORTING STEEL DECK IS 100MM BELOW TOP OF CONCRETE.
- COORDINATE CONNECTIONS OF P.C. PANELS TO STEEL BEAMS/COLUMNS WITH P.C. SUPPLIER.
- SEE ALSO GENERAL NOTES AND TYPICAL DETAILS ON DRAWINGS S501 & S502.

DESIGN LOADS FOR SECOND FLOOR JOISTS:

DEAD LOAD = 3.86 KPa
LIVE LOAD = 4.80 KPa
TOTAL LOAD = 8.66 KPa.

- LIVE LOAD DEFLECTION OF FLOOR JOISTS SHALL NOT EXCEED L/480
- TOTAL LOAD DEFLECTION OF FLOOR JOISTS SHALL NOT EXCEED L/360

B/S102 - MID-LANDING FRAMING PLAN

No.	Description	Date
2	ISSUED FOR BUILDING PERMIT	JUNE 17, 24
1	ISSUED FOR 50% PROGRESS	MAY 30, 24

CONFIRM ALL GRIDS, DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.
CONTACT ARCHITECT AND ENGINEER FOR ANY DISCREPANCIES.
ONLY LATEST APPROVED DRAWINGS TO BE USED FOR CONSTRUCTION.
ALL DRAWINGS REMAIN THE PROPERTY OF THE CONSULTANT.

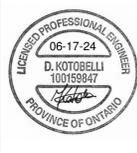


AMR ENGINEERING LTD.
STRUCTURAL ENGINEERS
920 Alness Street, Suite 205,
Toronto, ON M3J 2H7
(416) 551-1611

Project North



Stamp:



Project :

WAREHOUSE AND OFFICE HEADQUARTERS

45 BLOWERS CRES
AJAX, ON L1Z 0N4

Drawing Title:

SECOND FLOOR FRAMING PLAN

Date: 30-05-2024

Drawing No.

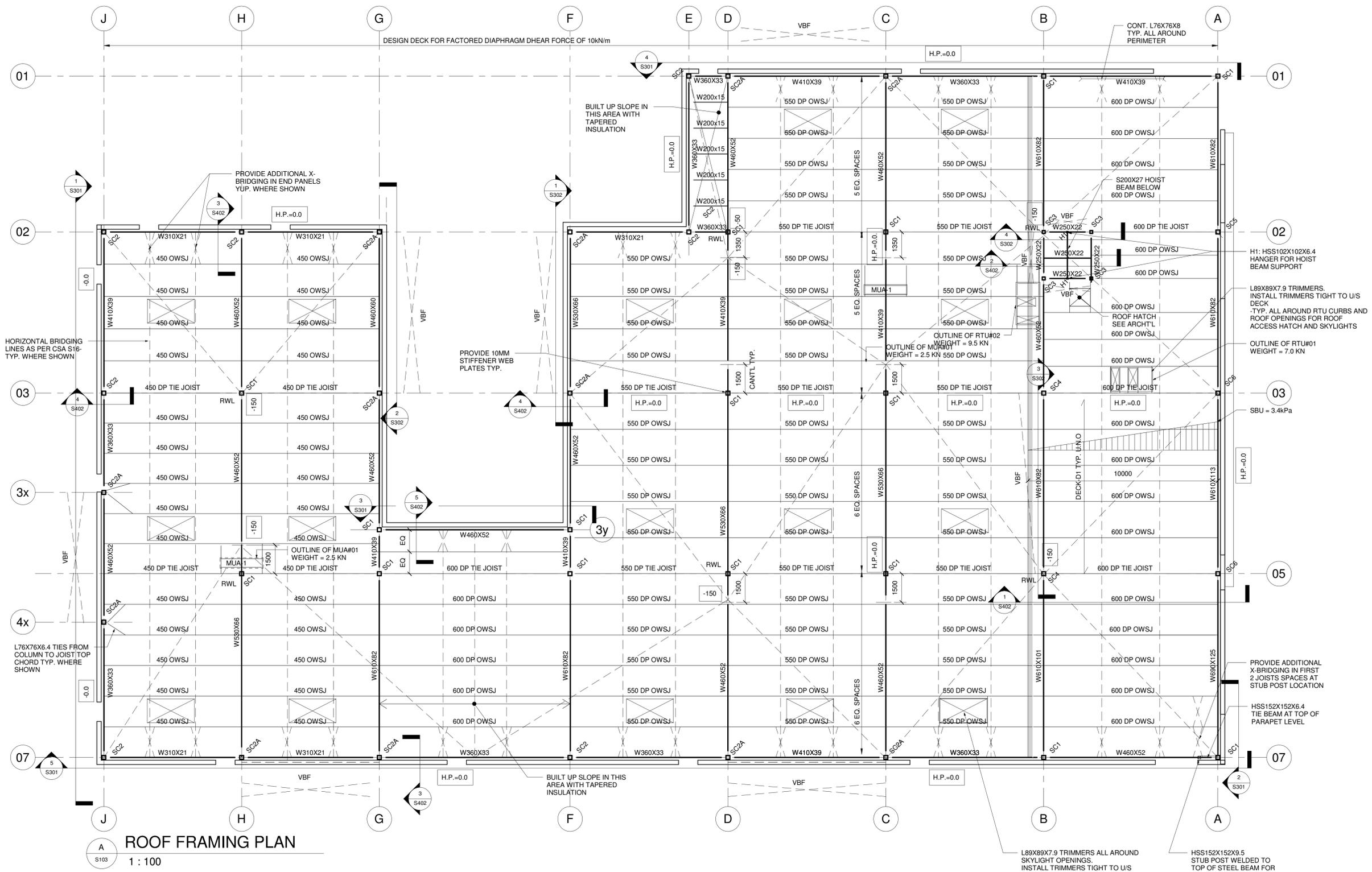
Scale: As Noted

S102

Drawn By: OH

Checked By: DK

Job No: 2280



ROOF FRAMING PLAN
1 : 100

- NOTES:**
- TOP OF STEEL DECK AT ROOF HIGH POINT (0.0) ON PLAN IS +8572 MM ABOVE GROUND FLOOR UNLESS NOTED OTHERWISE ON PLAN AND/OR SECTIONS.
 - TOP OF OWSJ IS 38MM BELOW TOP OF STEEL DECK. TOP OF STEEL BEAMS SUPPORTING OWSJ IS 100MM BELOW TOP OF OWSJ U.N.O. ON PLAN OR SECTIONS.
 - TOP OF STEEL BEAMS SUPPORTING STEEL DECK IS 38MM BELOW TOP OF DECK.
 - COORDINATE CONNECTIONS OF PC. PANEL TO STEEL BEAMS/COLUMNS WITH PC. SUPPLIER.
 - SEE ARCH'TL DRAWINGS FOR DIMENSIONS, ELEVATIONS AND ROOF SLOPES.
 - SEE ALSO GENERAL NOTES AND TYPICAL DETAILS ON DRAWINGS S501 & S502.

- DESIGN LOADS FOR ROOF JOISTS.**
- DEAD LOAD = 1.20 kPa. + MECH'L
LL (SNOW LOAD) = 1.20 kPa. + SBU
TOTAL LOAD = 2.40 kPa. + MECH'L + SBU
- NOTES:**
- ADD MECHANICAL LOADS WHERE SHOWN ON PLAN.
 - COORDINATE LOCATION OF ROOF TOP UNITS WITH MECHANICAL.
 - DESIGN ALL JOISTS FOR AN ADDITIONAL LOAD ON 1.8kN AT ANY LOCATION ALONG THE SPAN.
 - DESIGN AND CONNECT ALL ROOF JOISTS FOR WIND UPLIFT AS PER WIND DIAGRAM ON THIS DRAWING
 - ALL LOADS ARE SPECIFIED (UNFACTORED) LOADS. LIVE LOAD DEFLECTION < L/360.

NOTE 'A':
ALL INSULATED PRECAST PANELS ARE TO BE SELF SUPPORTING. CONNECT TO STRUCTURE FOR LATERAL SUPPORT ONLY TYP. UNO.

NOTE 'B':
ALL EXTERIOR STEEL FRAMING EXPOSED TO ELEMENTS SHALL BE HOT DIPPED GALVANIZED AS PER CSA-G164-18.

L89X89X7.9 TRIMMERS ALL AROUND SKYLIGHT OPENINGS. INSTALL TRIMMERS TIGHT TO U/S DECK - TYP. LOCATE SKYLIGHTS BETWEEN OWSJ. REFER TO ARCH DRAWINGS FOR ALL SKYLIGHT LOCATIONS AND DETAILS.

HSS152X152X9.5 STUB POST WELDED TO TOP OF STEEL BEAM FOR PERCAST LATERAL SUPPORT

No.	Description	Date
1	ISSUED FOR 50% PROGRESS	MAY 30,24

CONFIRM ALL GRIDS, DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.

CONTACT ARCHITECT AND ENGINEER FOR ANY DISCREPANCIES.

ONLY LATEST APPROVED DRAWINGS TO BE USED FOR CONSTRUCTION.

ALL DRAWINGS REMAIN THE PROPERTY OF THE CONSULTANT.



AMR ENGINEERING LTD.
STRUCTURAL ENGINEERS
920 Alness Street, Suite 205,
Toronto, ON M3J 2H7
(416) 551 - 1611

Project North

Stamp:



Project : **WAREHOUSE AND OFFICE HEADQUARTERS**

45 BLOWERS CRES
AJAX, ON L1Z 0N4

Drawing Title:
ROOF FRAMING PLAN

Date: 30-05-2024	Drawing No.
Scale: As Noted	S103
Drawn By: OH	
Checked By: DK	Job No: 2280

STRIP FOOTING SCHEDULE		
MARK	SIZE	NOTES
SF1	600 X 250 DP. + 3-15M CONT.	PROVIDE 15M DOWELS TO FOUNDATION WALL ABOVE (SEE SECTIONS)
SF2	500X 250 DP. + 2-15M CONT.	PROVIDE 15M DOWELS TO FOUNDATION WALL ABOVE (SEE SECTIONS)

SPREAD FOOTING SCHEDULE		
MARK	SIZE	NOTES
F1	1250X1250X300 DP. RW/ 5-15M B.E.W.	PROVIDE DOWELS TO MATCH PIER VERTICALS (SEE SECTIONS)
F2	1000X1000X300 DP. RW/ 4-15M B.E.W.	PROVIDE DOWELS TO MATCH PIER VERTICALS (SEE SECTIONS)
F3	1500X1500X350 DP. RW/ 6-15M B.E.W.	PROVIDE DOWELS TO MATCH PIER VERTICALS (SEE SECTIONS)
F4	1750X1750X350 DP. RW/ 7-15M B.E.W.	PROVIDE DOWELS TO MATCH PIER VERTICALS (SEE SECTIONS)
F5	2000X2000X400 DP. RW/ 8-15M B.E.W.	PROVIDE DOWELS TO MATCH PIER VERTICALS (SEE SECTIONS)
F6	1650X1650X350 DP. RW/ 6-15M B.E.W.	PROVIDE DOWELS TO MATCH PIER VERTICALS (SEE SECTIONS)
F7	2200X2200X450 DP. RW/ 7-20M B.E.W.	PROVIDE DOWELS TO MATCH PIER VERTICALS (SEE SECTIONS)
F8	2400X2400X450 DP. RW/ 8-20M B.E.W.	PROVIDE DOWELS TO MATCH PIER VERTICALS (SEE SECTIONS)
F9	2000X2000X400 DP. RW/ 8-15M T&B E.W.	PROVIDE DOWELS TO MATCH PIER VERTICALS (SEE SECTIONS)
F10	1200X1200X300 DP. RW/ 5-15M T&B E.W.	PROVIDE DOWELS TO MATCH PIER VERTICALS (SEE SECTIONS)

PIER SCHEDULE		
MARK	SIZE	PIER CONFIGURATION
CP1	700Ø CONC. PIER + 8-20M VERTICALS + 10M@300 TIES + 2 TIES AT TOP	
CP2	600X650 CONC. PIER + 8-20M VERTICALS + 3-10M@300 TIES + 2 TIES AT TOP	
CP2A	650X650 CONC. PIER + 8-20M VERTICALS + 3-10M@300 TIES + 2 TIES AT TOP	
CP3	600X600 CONC. PIER + 8-20M VERTICALS + 3-10M@300 TIES + 2 TIES AT TOP	
CP4	250 DIA. PIER (UNREINFORCED) 150 ABOVE GARDE	

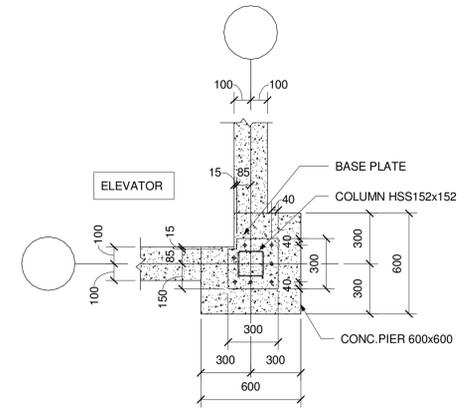
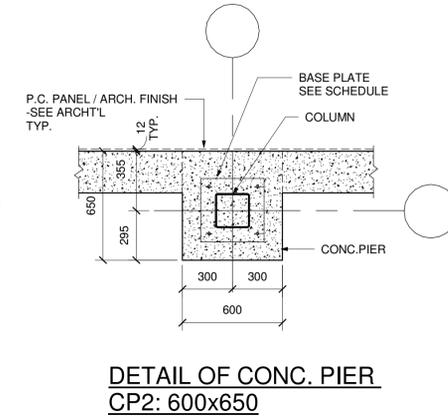
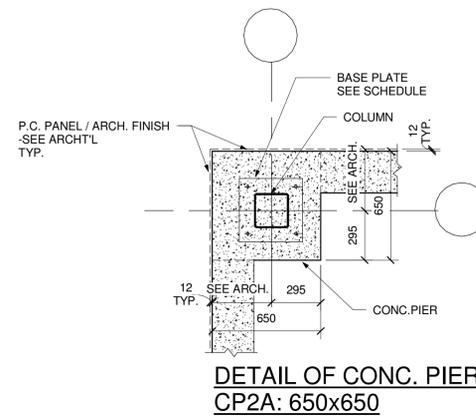
NOTES:
 1. TOP OF PIERS IS 250MM BELOW FINISH FLOOR ELEVATION TYP. U.N.O.
 2. TOP ON INTERIOR PIERS AT R.W.L. IS 450MM BELOW FINISH FLOOR ELEVATION TYP. U.N.O.

STEEL COLUMN SCHEDULE				
MARK	SIZE	BASEPLATE	ANCHOR BOLTS	BASEPLATE DETAIL
SC1	H55 203X203X8.0	380X25X380	4- 25Ø X 600 LONG. ANCHOR BOLTS (75 HOOK)	
SC2	H55 203X203X6.4	380X20X380	4- 25Ø X 600 LONG. ANCHOR BOLTS (75 HOOK)	
SC2A	H55 203X203X6.4	380X25X380	8- 25Ø X 600 LONG. HEADED ANCHOR BOLTS.	
SC3	H55 152X152X6.4	320X20X320	4- 25Ø X 600 LONG. ANCHOR BOLTS (75 HOOK)	
SC4	H55 203X203X8.0	380X25X380	8- 25Ø X 600 LONG. HEADED ANCHOR BOLTS	
SC5	H55 203X203X9.5	380X25X380	4- 25Ø X 600 LONG. ANCHOR BOLTS (75 HOOK)	
SC6	H55 203X203X13	380X25X380	4- 25Ø X 600 LONG. ANCHOR BOLTS (75 HOOK)	

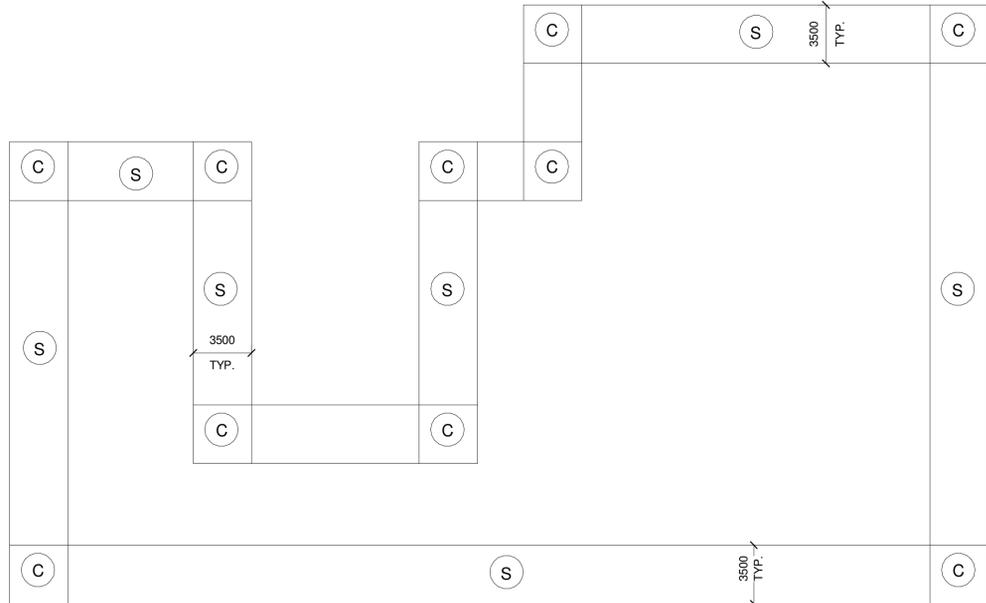
NOTES:
 1. UNDER ALL COLUMN BASE PLATE PROVIDE 6MM LEVELING PLATE AND 44MM NON-SHRINK GROUT. LEVELING PLATE SHALL PROJECT 12MM BEYOND COLUMN BASE PLATE ALL AROUND.
 2. ALL EXTERIOR STEEL COLUMN'S BASE PLATE, ANCHOR BOLTS ETC. SHALL BE HOT DIPPED GALVANIZED DURING STEEL FABRICATION.

STEEL DECK SCHEDULE		
MARK	SIZE	NOTES
D1	38MM STEEL DECK - MIN 22 GA. (0.76mm)	- MIN. 3 SPAN CONTINUOUS WHEREVER POSSIBLE - DESIGN DECK FOR A FACTORED DIAPHRAGM SHEAR FORCES SHOWN ON ROOF FRAMING PLAN
D2	62MM CONC. RW 152X152 MW/ 8.7 X MW/ 8.7 WWF + 38MM STEEL DECK - MIN 22 GA. (0.76MM COMPOSITE DECK)	- MIN. 3 SPAN CONTINUOUS

FOUNDATION WALL SCHEDULE		
MARK	SIZE	REINFORCING
FW1	250 CONCRETE WALL	2-15M TOP & BOTTOM & MIDDLE SEE ALSO PLAN, SECTIONS AND GENERAL NOTES.
FW2	250 CONCRETE WALL	15M@400 C/C V & H E.F. SEE ALSO PLAN, SECTIONS AND GENERAL NOTES.



CONC. PIER CP3 DETAIL



ROOF WIND UPLIFT (GROSS) DIAGRAM

R = 0.72 KPa.
 S = 1.20 KPa.
 C = 1.62 KPa

FOR CALCULATION OF NET UPLIFT ROOF
 DEAD LOAD = 0.72 KPa.

A
 S201

ROOF WIND UPLIFT DIAGRAM

1 : 200

Consultant:

ARCHITECT:
 SARPLY ARCHITECTS INC.
 80 ST. CLAIR AVE E. SUITE 806
 TORONTO, ONTARIO M4T 1N5
 P. 905.510.0595

No.	Description	Date
2	ISSUED FOR BUILDING PERMIT	JUNE 17, 24
1	ISSUED FOR 50% PROGRESS	MAY 30, 24

CONFIRM ALL GRIDS, DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.

CONTACT ARCHITECT AND ENGINEER FOR ANY DISCREPANCIES.

ONLY LATEST APPROVED DRAWINGS TO BE USED FOR CONSTRUCTION.

ALL DRAWINGS REMAIN THE PROPERTY OF THE CONSULTANT.



AMR ENGINEERING LTD.
 STRUCTURAL ENGINEERS
 920 Alness Street, Suite 205,
 Toronto, ON M3J 2H7
 (416) 551 - 1611

Project North

Stamp:



Project :

WAREHOUSE AND
 OFFICE HEADQUARTERS

45 BLOWERS CRES
 AJAX, ON L1Z 0N4

Drawing Title:

SCHEDULES & PIER
 DETAILS

Date: 30-05-2024

Drawing No.

Scale: As Noted

S201

Drawn By: Author

Checked By: Checker

Job No: 2280

Consultant:

ARCHITECT:

SARPLY ARCHITECTS INC.
80 ST. CLAIR AVE E. - SUITE 806
TORONTO, ONTARIO M4T 1N5
P. 905.510.0595



- T/O FEATURE PARAPET 12278
- T/O HIGH PARAPET 10363
- T/O LOW PARAPET 8992
- ROOF 8495 8534
- 2nd FLOOR 3962
- GROUND FLOOR 0

No.	Description	Date
2	ISSUED FOR BUILDING PERMIT	JUNE 17, 24
1	ISSUED FOR 50% PROGRESS	MAY 30, 24

CONFIRM ALL GRIDS, DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.
CONTACT ARCHITECT AND ENGINEER FOR ANY DISCREPANCIES.
ONLY LATEST APPROVED DRAWINGS TO BE USED FOR CONSTRUCTION.
ALL DRAWINGS REMAIN THE PROPERTY OF THE CONSULTANT.



AMR ENGINEERING LTD.
STRUCTURAL ENGINEERS
920 Alness Street, Suite 205,
Toronto, ON M3J 2H7
(416) 551 - 1611

Project North

Stamp:



Project :

WAREHOUSE AND OFFICE HEADQUARTERS

45 BLOWERS CRES
AJAX, ON L1Z 0N4

Drawing Title:

ELEVATIONS

Date: 30-05-2024

Drawing No.

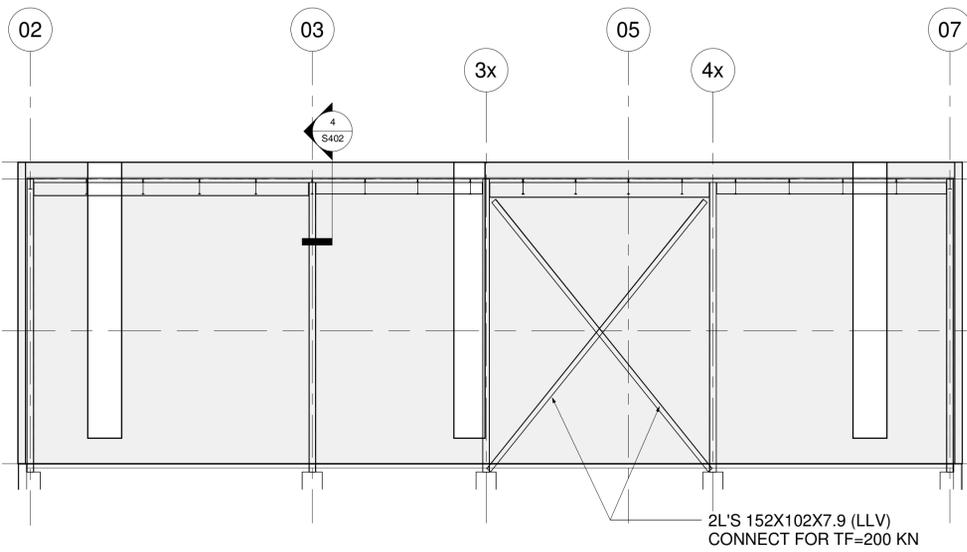
Scale: As Noted

S301

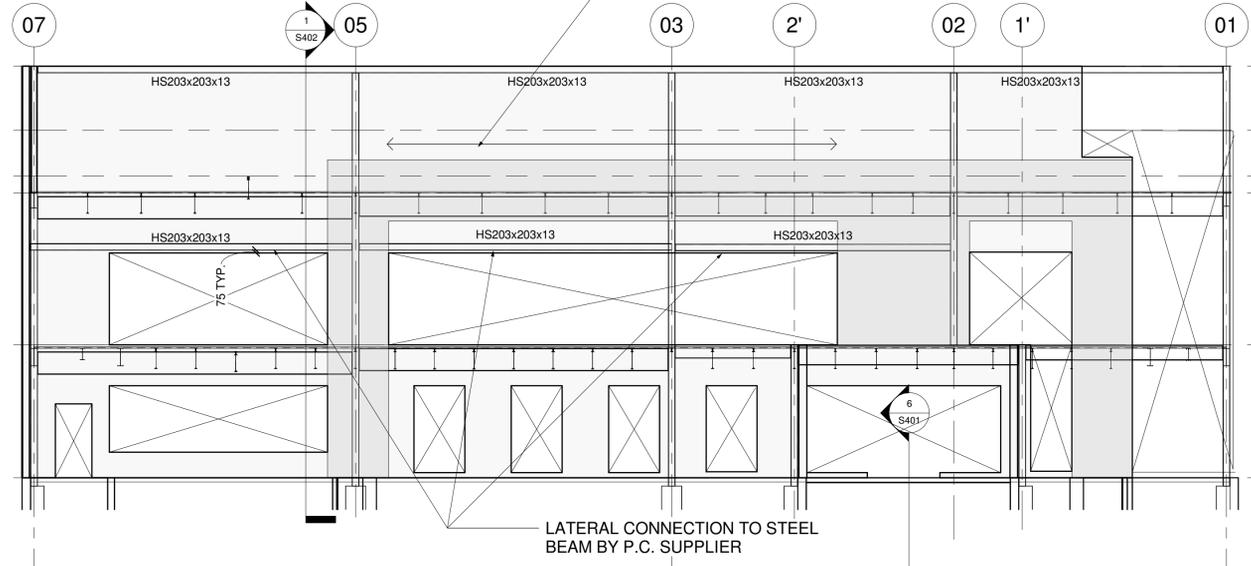
Drawn By: Author

Checked By: Checker

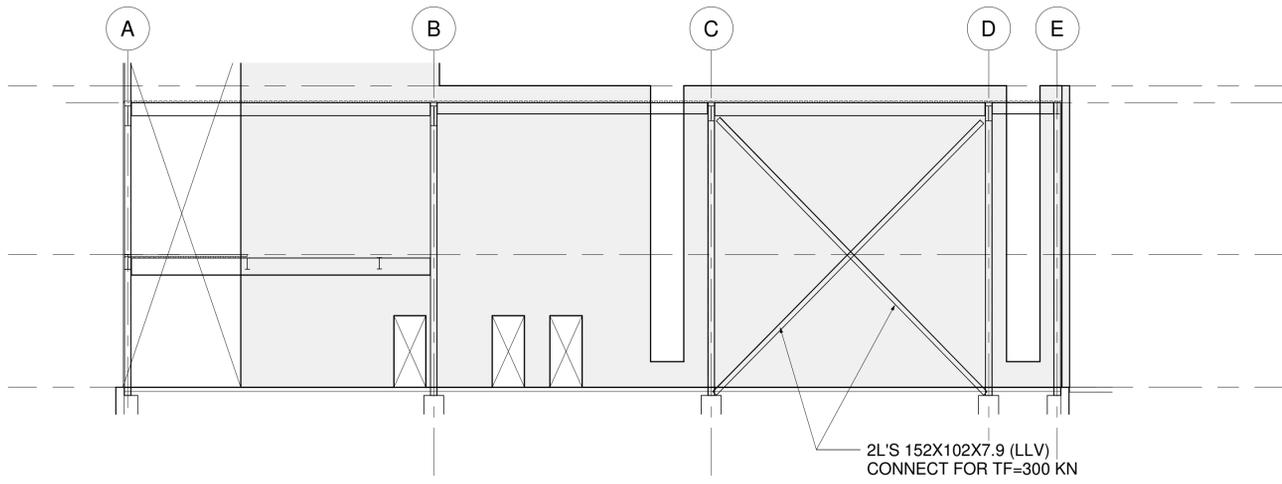
Job No: 2280



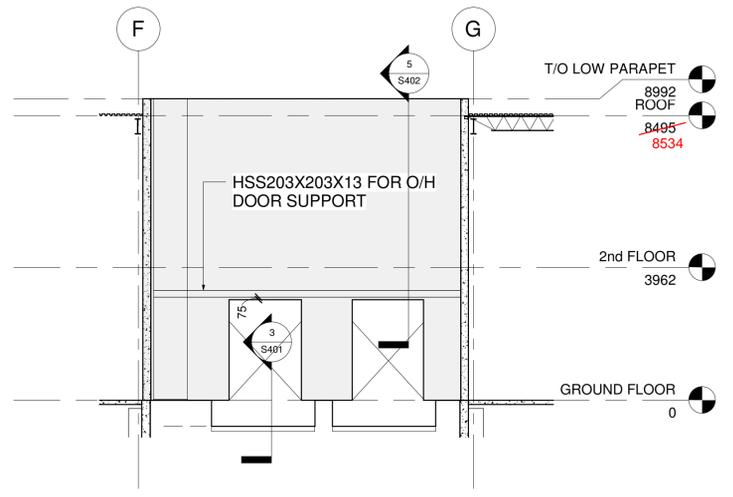
1
S301
ELEVATION ON GRID J (WEST)
1 : 100



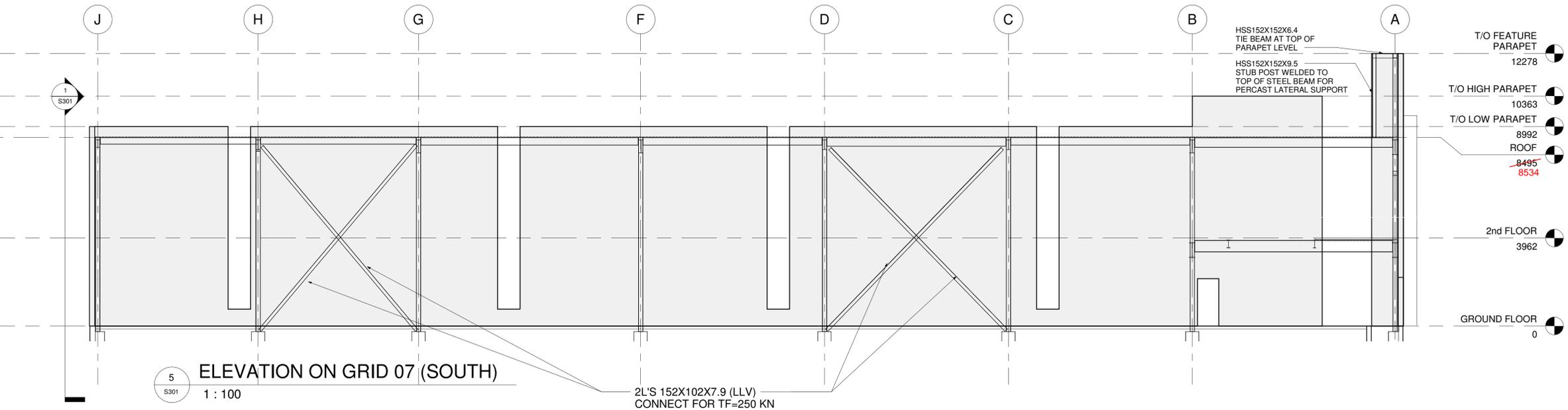
2
S301
ELEVATION ON GRID A (EAST)
1 : 100



4
S301
PART ELEVATION ON GRID 01 (NORTH)
1 : 100



3
S301
PART ELEVATION LOADING DOCK (NORTH)
1 : 100



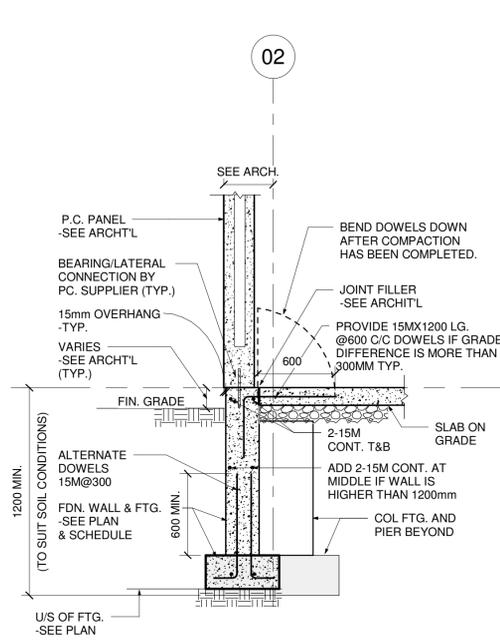
5
S301
ELEVATION ON GRID 07 (SOUTH)
1 : 100

2L'S 152X102X7.9 (LLV)
CONNECT FOR TF=250 KN

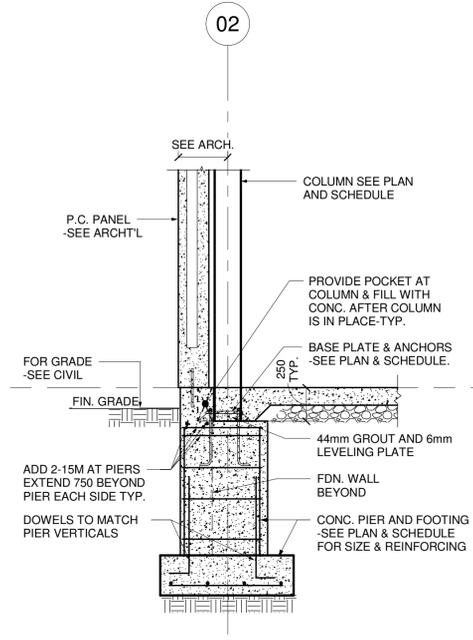
Consultant:

ARCHITECT:

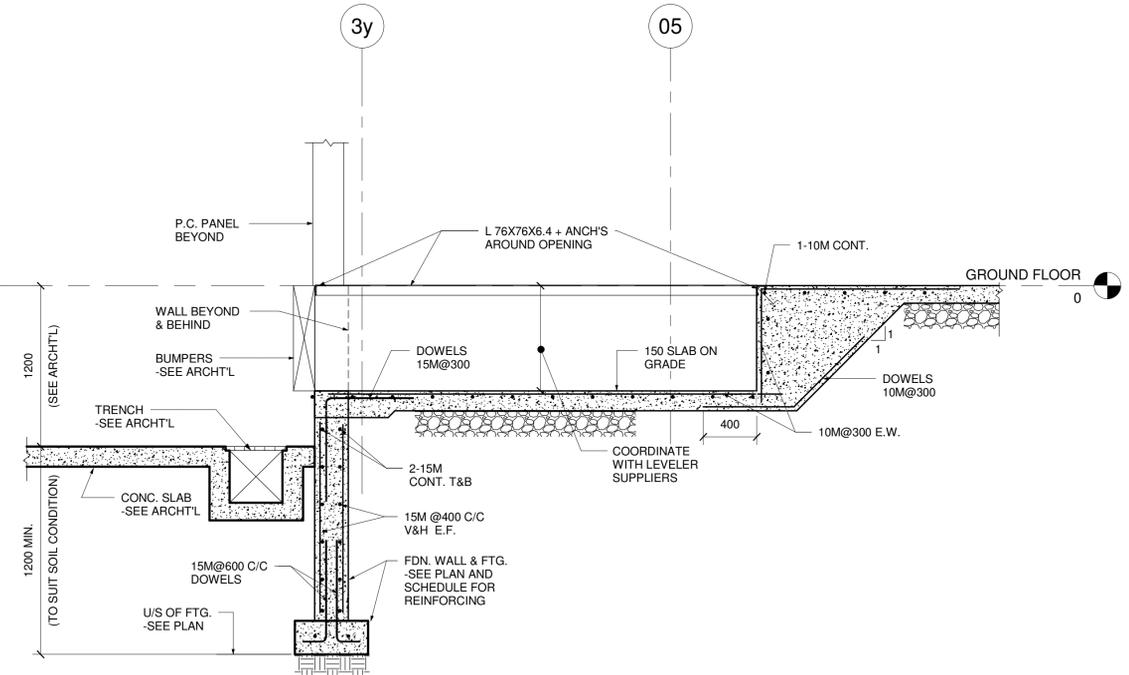
SAPLYS ARCHITECTS INC.
80 ST. CLAIR AVE E., SUITE 806
TORONTO, ONTARIO M4T 1N5
P. 905.510.0595



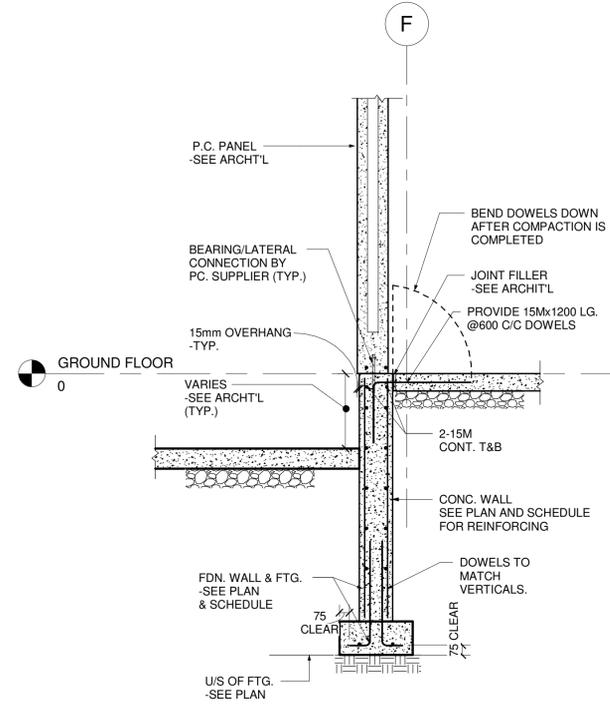
1 SECTION
S401 1 : 25



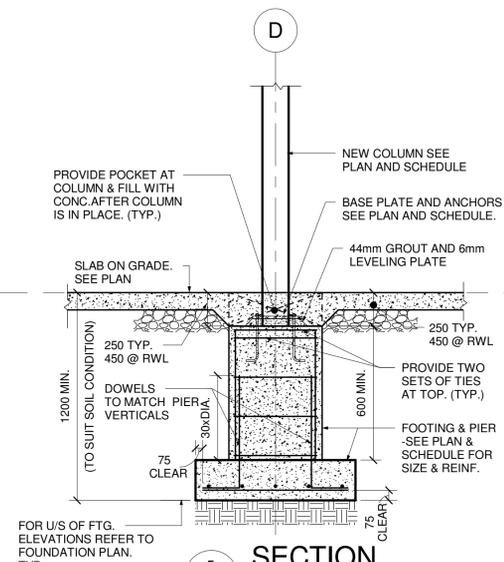
2 SECTION
S401 1 : 25



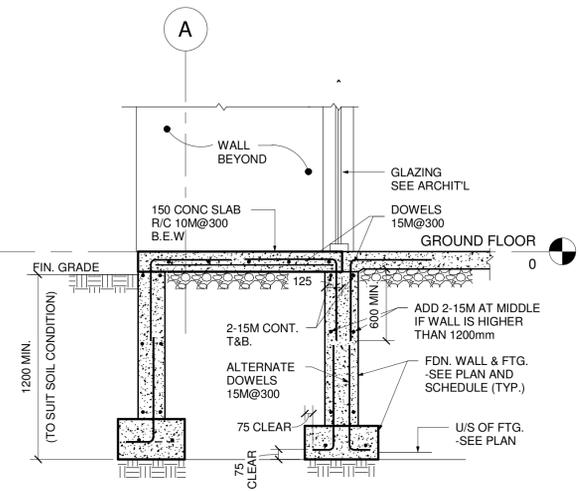
3 SECTION
S401 1 : 25



4 SECTION
S401 1 : 25



5 SECTION
S401 1 : 25



6 SECTION
S401 1 : 25

No.	Description	Date
2	ISSUED FOR BUILDING PERMIT	JUNE 17, 24
1	ISSUED FOR 50% PROGRESS	MAY 30, 24

CONFIRM ALL GRIDS, DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.
CONTACT ARCHITECT AND ENGINEER FOR ANY DISCREPANCIES.
ONLY LATEST APPROVED DRAWINGS TO BE USED FOR CONSTRUCTION.
ALL DRAWINGS REMAIN THE PROPERTY OF THE CONSULTANT.



Project North

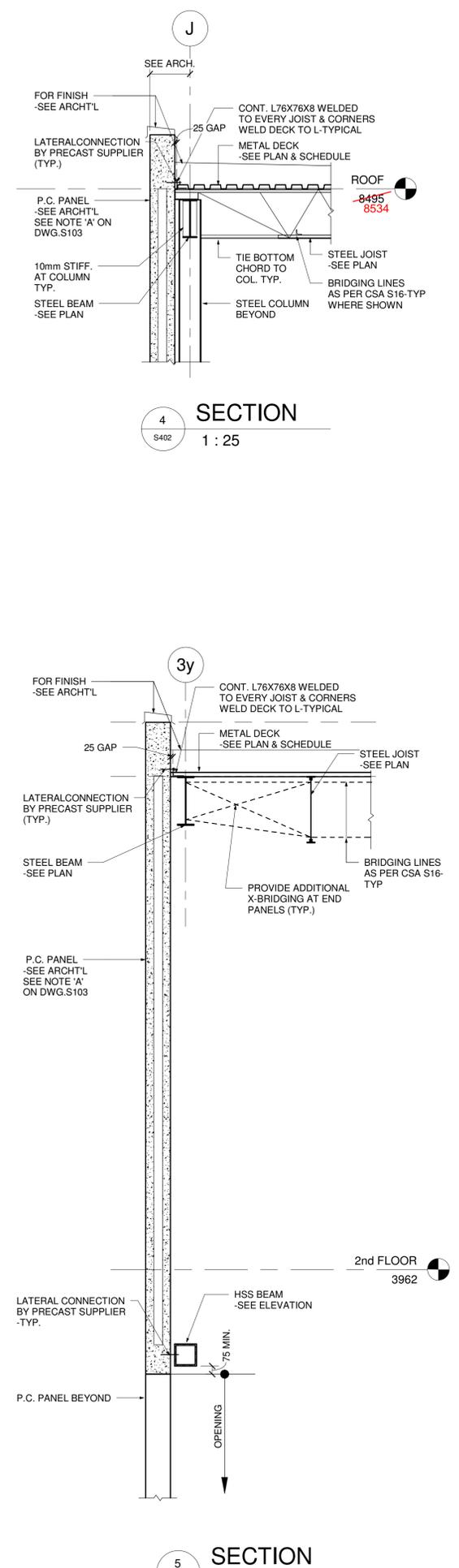
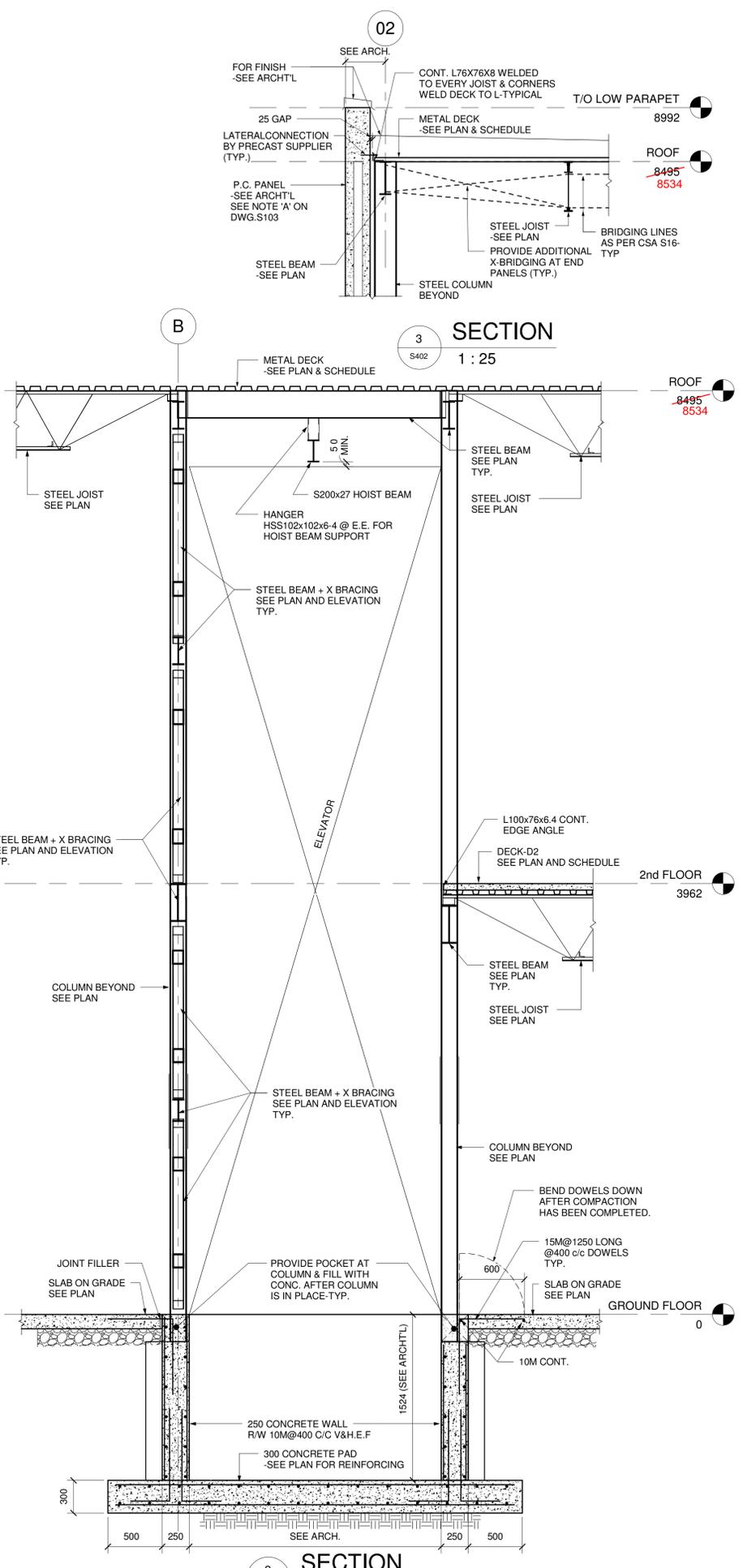
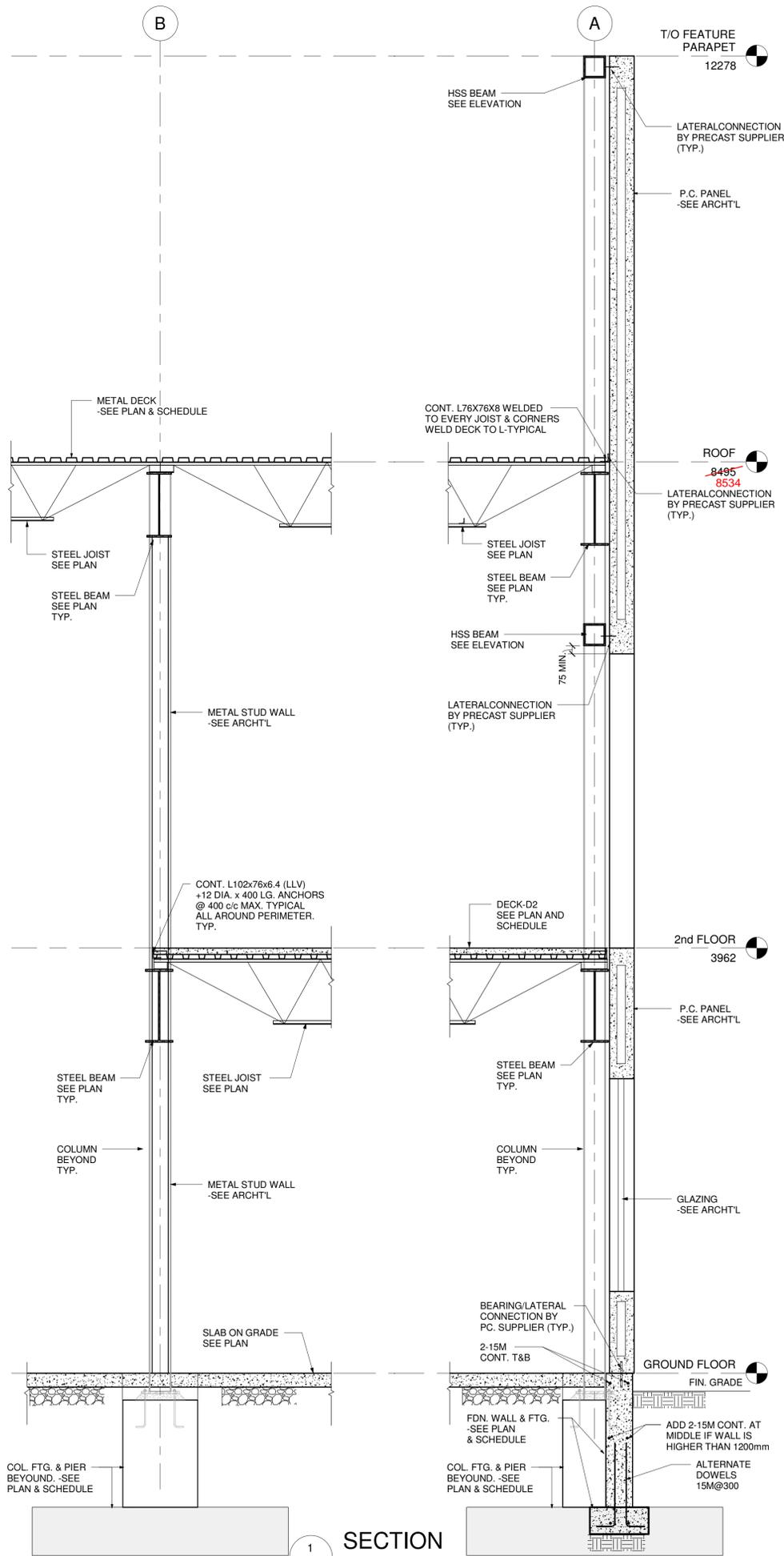
Stamp:

Project :
WAREHOUSE AND OFFICE HEADQUARTERS

45 BLOWERS CRES
AJAX, ON L1Z 0N4

Drawing Title:
SECTIONS

Date: 06/08/21	Drawing No.
Scale: As Noted	S401
Drawn By: Author	
Checked By: Checker	Job No: 2280



Consultant:

ARCHITECT:
SAPLYS ARCHITECTS INC.
80 ST. CLAIR AVE E., SUITE 806
TORONTO, ONTARIO M4T 1N5
P. 905.510.0595

SAI
SAPLYS
ARCHITECTS

No.	Description	Date
1	ISSUED FOR BUILDING PERMIT	JUNE 17, 24
Issues / Revision		

CONFIRM ALL GRIDS, DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.
CONTACT ARCHITECT AND ENGINEER FOR ANY DISCREPANCIES.
ONLY LATEST APPROVED DRAWINGS TO BE USED FOR CONSTRUCTION.
ALL DRAWINGS REMAIN THE PROPERTY OF THE CONSULTANT.

AMR
AMR ENGINEERING LTD.
STRUCTURAL ENGINEERS
920 Alness Street, Suite 205,
Toronto, ON M3J 2H7
(416) 551 - 1611

Project North
Stamp:
LICENSED PROFESSIONAL ENGINEER
06-17-24
D. KOTOBELLI
100159847
PROVINCE OF ONTARIO

Project :
WAREHOUSE AND OFFICE HEADQUARTERS
45 BLOWERS CRES
AJAX, ON L1Z 0N4

Drawing Title:
SECTIONS

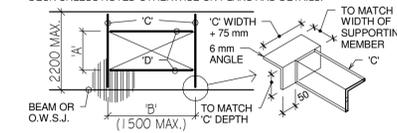
Date: 06/01/24
Scale: As Noted
Drawing No. **S402**
Drawn By: Author
Checked By: Checker
Job No: 2280

STRUCTURAL STEEL

- STRUCTURAL STEEL SECTIONS SHALL BE NEW AND CONFORM TO THE FOLLOWING:
 - WIDE FLANGE BEAMS AND W/F/ SECTIONS --- CSA G40.21 350W
 - MISCELLANEOUS ROLLED SECTIONS (EXCEPT WIDE FLANGES) --- CSA G40.21 300W
 - HOLLOW STRUCTURAL SECTIONS (CLASS C U.N.O.) --- CSA G40.21 350W
 - ROLLED PLATES --- CSA G40.21 300W
 - BOLTS (SEE PLANS AND DETAILS) --- ASTM A325 OR ASTM A490
 - STRUCTURAL STEEL ANCHOR RODS (U.N.O.) --- ASTM F1554 GRADE 36 MINIMUM CAN/CSA S30.18R, GRADE 400
 - REINFORCING BAR ANCHOR BOLTS --- CAN/CSA S30.18R, GRADE 400
- ALL CONNECTIONS TO BE DESIGNED BY FABRICATOR UNLESS NOTED OTHERWISE. ALL BEAM CONNECTIONS TO BE STANDARD FRAME BEAM CONNECTIONS OR EQUIVALENT. UNLESS NOTED OTHERWISE, SUBMIT A LETTER OF CERTIFICATION BY P.ENG RESPONSIBLE FOR DESIGN OF CONNECTIONS.
- SHOP DRAWINGS SHALL BE PREPARED UNDER THE DIRECTION OF A SPECIALTY STRUCTURAL ENGINEER. FOR THOSE CONNECTIONS AND COMPONENTS DESIGNED BY THE FABRICATOR, THIS ENGINEER OR THEIR REPRESENTATIVE SHALL VISIT THE SITE TO REVIEW IN PLACE THE CONNECTIONS AND COMPONENTS DESIGNED BY THIS ENGINEER TO SATISFY THEMSELVES THAT THESE CONNECTIONS AND COMPONENTS COMPLY WITH THEIR DESIGN ON THE SHOP DRAWINGS. THIS ENGINEER SHALL PROVIDE A LETTER TO AMR TO THIS EFFECT. THIS ENGINEER SHALL ALSO PROVIDE SEALED SKETCHES FOR ALL FIELD MODIFICATIONS MADE TO THEIR DESIGN.
- SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO START OF STEEL FABRICATION.
- FABRICATION, ERECTION, STRUCTURAL DESIGN, AND DETAILING OF ALL STEEL SHALL BE IN ACCORDANCE WITH CAN/CSA-S16.
- FILLET WELDS SHALL BE 5 mm MINIMUM U.N.O.
- BOLTS SHALL BE A325 19 mm Ø MINIMUM U.N.O.
- BOLTED CONNECTIONS SHALL HAVE A MINIMUM OF TWO BOLTS IN EACH MEMBER U.N.O.
- UNLESS NOTED OTHERWISE, COLUMN CAP PLATES SHALL BE 16 mm THICK AND COLUMN BASE PLATES SHALL BE 20 mm MINIMUM THICK.
- PROVIDE 6 mm CAP PLATES FOR ALL HSS MEMBERS U.N.O.
- CONNECTION DETAILS SHOWN ON THE STRUCTURAL DRAWINGS SHALL NOT BE ALTERED BY THE CONTRACTOR WITHOUT WRITTEN APPROVAL FROM AMR ENGINEERING LIMITED.
- UNLESS NOTED OTHERWISE ON THE PLANS, REFER TO THE DETAILS IN THE GENERAL NOTES FOR FRAMING FOR SUPPORT OF ROOF TOP MECHANICAL EQUIPMENT.
- STEEL TO BE EXPOSED IN FINISHED WORK SHALL BE CLEANED, PREPARED, PRIMED AND PAINTED IN ACCORDANCE WITH CSA STANDARD S16 AND THE ARCHITECTURAL DRAWINGS AND PAINTING SPECIFICATION.
- DESIGN DRAWINGS INCLUDE ARCHITECTURAL, MECHANICAL, AND ELECTRICAL DRAWINGS. SEE ALSO ARCHITECTURAL DRAWINGS FOR ROOF AND FLOOR ELEVATIONS, ROOF SLOPES, EDGE DETAILS, AND ADDITIONAL DIMENSIONS AND DETAILS. WHERE ELEVATIONS, ROOF SLOPES, ETC., ARE SHOWN ON THE STRUCTURAL DRAWINGS, THEY MUST BE CONFIRMED WITH THE ARCHITECTURAL DRAWINGS.
- UNLESS NOTED OTHERWISE, DO NOT OVERSIZE HOLES IN STEEL TO FIT ANY ANCHOR LOCATIONS. FOLLOW BASE PLATE HOLES. UNLESS NOTED OTHERWISE ON DRAWINGS, FOLLOW STANDARD PRACTICE WHICH IS TO USE SLIGHTLY OVERSIZED HOLES. USE 6 mm OVERSIZED HOLE DIAMETER FOR COLUMN ANCHOR RODS UP TO AND INCLUDING 27 mm DIAMETER, AND 12 mm OVERSIZED HOLE DIAMETER FOR COLUMN ANCHOR RODS GREATER THAN 27 mm DIAMETER.
- TOUCH UP ALL FIELD WELDS. ALL STEEL SHALL BE PAINTED WITH 1 ZINC RICH SHOP COAT AND FIELD TOUCH UP AS PER CGSB-140-M89. ALL EXTERIOR STEEL EXPOSED TO ELEMENTS SHALL BE HOT DIPPED GALVANIZED AS PER REQUIREMENTS OF CSA G184-18.
- NON-SHRINK GROUT SHALL BE M-BED STANDARD BY SIKKA CANADA INC. OR APPROVED EQUAL.

CONNECTION ANGLES FOR MAXIMUM FACTORED VERTICAL LOAD OF 6 kN.

TYPICAL FRAMING AROUND OPENINGS IN STEEL DECK WITHOUT CONCRETE TOPPING - U.N.O.

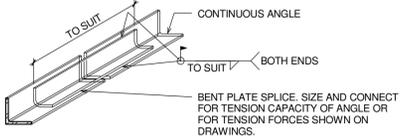
- SEE ALSO MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS FOR ALL OPENINGS IN DECK.
- U.N.O. REINFORCE OPENINGS WITHOUT MECHANICAL UNITS BETWEEN 150 mm TO 450 mm MAXIMUM DIMENSION WITH L76x76x4.8 X 1200 mm LONG, WELD TO EVERY FLUTE. I.E. 
- TYPICAL DETAILS FOR SMALL MECHANICAL UNITS AND/OR OPENINGS IN DECK UNLESS NOTED OTHERWISE ON PLANS AND DETAILS: 

MECH. UNIT OR OPENING SIZE 'A' x 'B'	ANGLE 'C'	ANGLE 'D'	MECHANICAL UNIT WEIGHT (SPECIFIED)
150 X 150 TO 450 X 450	L76x76x4.8	L76x76x4.8	0.25 kN TO 1.0 kN
450 X 450 TO 1500 X 1500	L102x102x6.4	L76x76x4.8	NO UNIT
450 X 450 TO 1500 X 900	L102x102x6.4	L76x76x4.8	LESS THAN OR EQUAL TO 2 kN (1500 mm MAX. HIGH)
450 X 900 TO 1500 X 1500	L102x102x6.4	L102x102x6.4	LESS THAN OR EQUAL TO 2 kN (1500 mm MAX. HIGH)

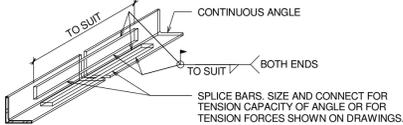
- WHERE AN OPENING IN DECK IS UNDER A MECHANICAL UNIT AND IS SMALLER THAN THE FRAMING REQUIRED TO SUPPORT THE MECHANICAL UNIT, REINFORCE THE OPENING WITH L76x76x4.8 ON ALL FOUR SIDES SPANNING BETWEEN THE MECHANICAL UNIT SUPPORT MEMBERS. SEE NOTE 3, PLANS AND DETAILS FOR MECHANICAL UNIT SUPPORT FRAMING.

TYPICAL TENSION SPLICE FOR ANGLES

ALTERNATE 1



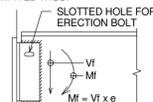
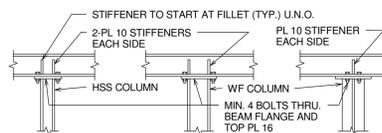
ALTERNATE 2



MINIMUM BEND RADIUS FOR STEEL PLATES

	"T" (PLATE THICKNESS)	"R" (MINIMUM INSIDE RADIUS)
	0 mm TO 6 mm	3 x "T"
	6 mm TO 12 mm	4 x "T"

STEEL BEAMS AND GIRDERS

- UNLESS NOTED, BEAM AND GIRDER CONNECTIONS TO EMBEDDED PLATES SHALL BE DOUBLE ANGLE FRAMING CONNECTIONS WELDED TO THE BEAM WEB THUS: 
- UNLESS NOTED OTHERWISE ALL CONNECTIONS FOR BEAMS AND GIRDERS SHALL BE DESIGNED FOR A SHEAR BASED TO THE MEMBER'S FULL MOMENT RESISTANCE CAPACITY RELATED TO A UNIFORM LOAD ON A SIMPLE SUPPORTED SPAN.
- STEEL BEAM CAMBERS SHOWN THUS  MEAN CAMBER BEAMS 75 mm AT CENTRE.
- TOP FLANGES OF BEAMS TO BE FREE OF ALL PAINT, DIRT, HEAVY RUST, MILL SCALE, SAND AND OTHER MATERIALS WHICH WILL INTERFERE WITH WELDING OF STUD SHEAR CONNECTIONS AND STEEL DECK TO BEAMS.
- UNLESS NOTED OTHERWISE WHERE BEAMS SIT OVER COLUMNS, PROVIDE FULL HEIGHT, FULL WIDTH PL 10 STIFFENERS EACH SIDE OVER COLUMN. 

STEEL DECK

- STEEL DECKING SHALL CONFORM TO CAN/CSA-S136.
- STEEL DECKING SHALL CONFORM TO CSSBI SPECIFICATION 10M MINIMUM GRADE 230 ZINC COATED STRUCTURAL QUALITY STEEL FOR ROOF AND FLOOR DECK. BASE STEEL NOMINAL THICKNESSES INDICATED ON THE DRAWINGS ARE MINIMUM REQUIREMENTS ONLY.
- INTERIOR EXPOSURE DECK SHALL BE ZINC COATED WIPE COAT ZP075 FOR FLOORS AND FOR ROOFS. EXTERIOR EXPOSURE DECK SHALL BE Z275 ZINC COATED UNLESS NOTED OTHERWISE.
- STEEL DECKING SHALL BE INSTALLED SUCH THAT SHEETS ARE SET FOR A MINIMUM OF THREE SPANS CONTINUOUS UNLESS NOTED OTHERWISE. LAPS OF DECKING SHALL BE LIMITED OR DETAILED TO PREVENT UNDUE VERTICAL DEFORMATIONS AT THE END OF THE DECK DUE TO END ROTATIONS.
- SEE DRAWINGS & NOTES FOR DECK THICKNESSES OR DESIGN LOADS.
- WHERE DECK IS CALLED UP ON THE DRAWINGS, ALTERNATES MUST BE THE SAME DEPTH, BE EQUIVALENT FOR DEFLECTIONS, VERTICAL LOAD, AND SHEAR CAPACITY, AND BE PRE-APPROVED.
- SUBMIT SHOP DRAWINGS INDICATING THE DECK SPANS, THICKNESSES, PROFILES, AND DETAILS. WHERE THE DECK THICKNESSES AND CONNECTIONS ARE NOT SHOWN ON THE DRAWINGS, THE FABRICATOR SHALL DESIGN THE DECK AND CONNECTIONS FOR THE VERTICAL LOADS AND THE SHEAR/DIAPHRAGM LOADS NOTED ON THE DRAWINGS AND HAVE THE SHOP DRAWINGS SEALED BY THE CONTRACTOR'S SPECIALTY STRUCTURAL ENGINEER.
- FASTENINGS - MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE ON DRAWINGS.
 - SIDE LAPS MECHANICALLY FASTENED (CLINCHED) AT NOT MORE THAN 600 mm O.C.
 - 20 mm Ø FUSION WELDS AT DECK SUPPORTS SHALL BE AT 300 mm O/C ALONG SIDE EDGES AND AT EVERY OTHER FLUTE.
- REMOVE WATER BETWEEN THE DECK AND SUPPORTING STEEL BEFORE WELDING DECK.
- IF NOT SHOWN OTHERWISE, ALL EDGES OF STEEL DECKING SHALL BE SUPPORTED ON CONTINUOUS ANGLE L76x76x6.4.
- SEE ALSO MECHANICAL, ELECTRICAL AND ARCHITECTURAL DRAWINGS FOR ALL OPENINGS IN DECK.
- STEEL DECK TO BE DESIGNED FOR A MINIMUM DIAPHRAGM SHEAR OF 4.0 kN/m, U.N.O.
- TOUCH UP ALL FIELD WELDS WITH ZINC RICH PAINT.

EXTERIOR WIND STEEL STUD NOTES

- SUBMIT SHOP DRAWINGS TO THE ARCHITECT FOR REVIEW BEFORE FABRICATION IS STARTED. ASSUME RESPONSIBILITY FOR THE ACCURACY OF THE WORK AND BE AWARE THAT REVIEW OF SHOP DRAWINGS IS ONLY TO ENSURE THAT THE CONTRACT DRAWINGS ARE BEING CORRECTLY INTERPRETED.
- SHOP DRAWINGS SHALL BEAR THE SEAL OF PROFESSIONAL ENGINEER OF ONTARIO RESPONSIBLE FOR DESIGN OF METAL STUDS.
- DESIGN STEEL STUDS FOR ANTICIPATED WIND LOADS LIVE AND DEAD LOADS IN ACCORDANCE WITH ONTARIO BUILDING CODE. REFER TO ARCHITECTURAL DRAWINGS FOR WALL CONSTRUCTION/TYPES. DEAD LOADS IN ACCORDANCE WITH ONTARIO BUILDING CODE.
- CONFORM TO THE REQUIREMENTS OF FIRE RATED ASSEMBLIES.
- DESIGN AND INSTALL BRIDGING TO PREVENT MEMBER ROTATION AND MEMBER TRANSLATION PERPENDICULAR TO THE MINOR AXIS.
- MAXIMUM DEFLECTION UNDER SPECIFIED LOADS SHALL NOT EXCEED THE FOLLOWING:
 - WALL STUDS SUPPORTING MASONRY VENEER - SPAN/480
 - WALL STUDS SUPPORTING OTHER FINISHES - SPAN/360
 - LINTEL MEMBERS - SPAN/480
- SPACING OF WALL STUDS & JOISTS SHALL NOT EXCEED 16" UNLESS NOTED OTHERWISE ON PLANS AND/OR SECTIONS.
- STEEL STUDS SHALL BE ROLL FORMED FROM ZINC COATED STEEL SHEETS CONFORMING TO ASTM A446-83 GRADE 'A' WITH ZINC COATING ON EACH SIDE. ZINC COATING DESIGNATION SHALL BE Z275.
- STEEL STUDS SHALL BE AS MANUFACTURED BY BAILEY METAL PRODUCT LIMITED OR AN APPROVED EQUAL WITH A MINIMUM METAL THICKNESS BEFORE GALVANIZING SHALL BE 1.27 mm (18 GAUGE). USE HEAVIER GAUGE AS REQUIRED BY DESIGN.
- FASTENERS SHALL BE CORROSION RESISTANCE PAN HEAD SELF DRILLING SELF TAPPING SCREWS.
- ERECT METAL STUDS TO TOLERANCE OF 1:1000.
- FRAME ALL OPENINGS IN STUD WALLS, BY USE OF ADDITIONAL FRAMING MEMBERS AND BRACING, TO CARRY ALL APPLICABLE LOADS ADEQUATELY.
- SPLICES IN STEEL STUDS WILL NOT BE PERMITTED.

STEEL DECK NOTES (WITH CONCRETE TOPPING)

- STEEL DECK TO BE AS NOTED ON DRAWINGS. UNLESS NOTED OTHERWISE ALL DECK RECEIVING CONCRETE TOPPING TO BE COMPOSITE DECK.
- IT IS INTENDED THAT ALL STEEL DECK BE UNSHORED DURING CONSTRUCTION U.N.O.
- THE STEEL DECK THICKNESS SHALL BE AS REQUIRED TO CARRY THE WET CONCRETE WITHOUT SHORING AND TO MEET THE LOADING REQUIREMENTS LISTED BELOW, AND WILL DEPEND ON THE ACTUAL PROFILE AND LAYOUT OF DECK USED. TOPPING THICKNESS ON THE DRAWINGS ARE MEASURED FROM THE TOP OF DECK FLUTE (SEE 'CONCRETE TOPPING ON STEEL DECK' NOTES).
- DESIGN FLOOR LOADS (SPECIFIED OR UNFACTORED):
CONSTRUCTION DEAD LOAD = WEIGHT OF WET CONCRETE
CONSTRUCTION LIVE LOAD = 1 kPa
SUPERIMPOSED DEAD LOAD = AS INDICATED ON DRAWINGS
SERVICE LIVE LOAD = AS INDICATED ON DRAWINGS
- ALL DECK TO BE THREE SPAN MINIMUM WHERE POSSIBLE.
- THE STEEL DECK PROFILES SHALL BE AS REQUIRED TO ACHIEVE FIRE SEPARATIONS AS SPECIFIED ON THE ARCHITECTURAL DRAWINGS. IN ADDITION, ALL COMPOSITE FLOOR DECK PROFILES SHALL HAVE AN AVERAGE BOTTOM FLUTE WIDTH AT LEAST 2 X DECK DEPTH WHERE USED ON COMPOSITE BEAMS WITH SHEAR STUDS.
- THE CONTRACTOR IS RESPONSIBLE FOR COORDINATING WITH SUBTRADES FOR PROVIDING STEEL DECK AS INDICATED ON THE DRAWINGS, AND ALL NECESSARY FORMING AT THE DECK EDGES FOR THE FULL DECK AND CONCRETE DEPTH TO PREVENT LEAKING OF THE CONCRETE TOPPING. THIS INCLUDES BUT IS NOT LIMITED TO ALL SLAB EDGES AT THE BUILDING EDGE AND ALL SLAB OPENINGS FRAMED BY STRUCTURAL STEEL INCLUDING ELEVATOR SHAFTS AND STAIRWELLS AND AROUND THE WEBS AND FLANGES OF ALL COLUMNS.
- BEAMS NOTED AS COMPOSITE ON THE DRAWINGS REQUIRE STUD SHEAR CONNECTIONS. SEE ALSO SHEAR CONNECTOR NOTES. SEE ALSO PLANS, SECTIONS, DETAILS AND SCHEDULES FOR STUDS SHOWN ON BEAMS/GIRDERS/DIAPHRAGM STRUTS ETC. OTHER THAN COMPOSITE BEAMS. THE CONTRACTOR SHALL CO-ORDINATE THE DESIGN, SUPPLY, AND INSTALLATION OF ALL STUDS.
- FASTENINGS - MINIMUM REQUIREMENTS UNLESS NOTED OTHERWISE ON DRAWINGS.
 - SIDE LAPS MECHANICALLY FASTENED (CLINCHED) AT NOT MORE THAN 600 mm O.C.
 - 20 mm Ø FUSION WELDS AT DECK SUPPORTS SHALL BE AT 300 mm O/C ALONG SIDE EDGES AND AT EVERY OTHER FLUTE.
- SEE ALSO MECHANICAL AND ARCHITECTURAL DRAWINGS FOR OPENINGS, HOLES ETC. IN DECKING.
- SUBMIT SHOP DRAWINGS SEALED BY A SPECIALTY STRUCTURAL ENGINEER FOR DESIGN OF DECK AND SHEAR STUDS.

OPEN WEB STEEL JOISTS

- OPEN WEB STEEL JOISTS (O.W.S.J.) SHALL CONFORM TO CAN/CSA-S16.
- DESIGN OF O.W.S.J. SHALL CONFORM TO CLAUSE 16 OF CAN/CSA-S16 (LIMIT STATES DESIGN OF STEEL STRUCTURES) AND CAN/CSA-S136 (DESIGN OF LIGHT GAUGE STEEL STRUCTURAL MEMBERS), BASED ON THE LOADINGS INDICATED ON THE DRAWINGS AND LISTED BELOW.
- O.W.S.J. SHALL BE DESIGNED FOR THE LOADS SHOWN ON THE DRAWINGS.
- IN ADDITION TO THE POINT LOADS CALLED FOR ON THE DRAWINGS AND IN THE GOVERNING BUILDING CODE, DESIGN O.W.S.J. FOR A 1.8 kN FACTORED ADDITIONAL POINT LOAD AT ANY LOCATION ON TOP CHORD AND BOTTOM CHORD (INCLUDING THE EFFECTS OF LOCAL BENDING) CONCURRENT WITH OTHER DESIGN LOADS. OVER MECHANICAL AREAS THE DESIGN LOADS SHALL BE 4.5 kN FACTORED. THE ADDITIONAL POINT LOADS ON EACH CHORD NEED NOT BE APPLIED CONCURRENTLY WITH EACH OTHER.
- UNLESS NOTED OTHERWISE LIVE LOAD DEFLECTION SHALL NOT EXCEED 1/360 OF THE SPAN.
- CAMBER ALL O.W.S.J. FOR DEAD LOAD PLUS 1/2 LIVE LOAD UNLESS OTHERWISE NOTED. PROVIDE A MINIMUM CAMBER OF 12 mm.
- DESIGN AND PROVIDE O.W.S.J. BRIDGING IN ACCORDANCE WITH CAN/CSA-S16 UNLESS OTHERWISE INDICATED ON THE DRAWINGS. REFER TO THE DRAWINGS FOR AREAS OF NON-TYPICAL O.W.S.J. BRIDGING AND BRACING.
- SUBMIT SHOP DRAWINGS FOR REVIEW PRIOR TO START OF O.W.S.J. FABRICATION. SHOP DRAWINGS SHALL BEAR THE SEAL OF THE SPECIALTY STRUCTURAL ENGINEER WHO IS RESPONSIBLE FOR THE DESIGN OF THE O.W.S.J. REFER TO THE SPECIFICATIONS.
- ALL O.W.S.J. TO HAVE BUILDING SERVICES PASS THROUGH THEM. WEB MEMBERS OF ADJACENT O.W.S.J. TO LINE UP TO ACCOMMODATE CONTINUOUS PENETRATION OF SERVICES.
- WELDING SHALL CONFORM TO CSA W59.
- O.W.S.J. TO BE PAINTED SHALL BE CLEANED AND SHALL RECEIVE ONE COAT OF SHOP PRIMER IN ACCORDANCE WITH CAN/CSA-S16.
- O.W.S.J. TO BE EXPOSED IN FINISHED WORK SHALL BE PAINTED WITH SHOP PRIMER MEETING THE REQUIREMENTS OF CAN/CSA-S16. CLEANING, PREPARATION OF STEEL AND THE PAINT PRODUCT SHALL BE COMPATIBLE WITH REQUIREMENTS OF FINISHED PAINTING AS SPECIFIED IN ARCHITECTURAL FINISHES. REFER TO THE SPECIFICATIONS.
- BOTTOM CHORD EXTENSIONS (B.C.E.) ARE EXTENSIONS OF THE BOTTOM CHORD WHICH TRANSMIT AN AXIAL FORCE TO EITHER A COLUMN, BEAM BOTTOM FLANGE, JOIST GIRDER BOTTOM CHORD OR WALL. THE EXTENSION MAY BE EITHER FLAT OR SLOPED. SEE PLANS, SCHEDULES, AND DETAILS FOR AXIAL FORCES.
- JOIST BEARING PLATES TO BE DESIGNED BY THE JOIST SUPPLIER AND SUBMITTED WITH THE JOIST SHOP DRAWING FOR OUR REVIEW. BEARING NOT TO EXCEED 100 PSI.
- DESIGN JOIST SHOES FOR ROLLING SHEAR = 6kN U.N.O.

MISC. METALS AND STEEL STAIRS

- PROVIDE SHOP DRAWINGS PRIOR TO FABRICATION STAMPED, SIGNED AND DATED BY P.ENG.
- ALL GUILDS TO BE DESIGNED TO MEET LATERAL LOAD DESCRIBED IN OBC 2012.
- ALL HANDRAILS TO BE DESIGNED TO MEET LATERAL LOAD DESCRIBED IN OBC 2012.
- ALL STAIRS TO BE DESIGNED TO SUPPORT A MINIMUM LIVE LOAD OF 4.8kPa

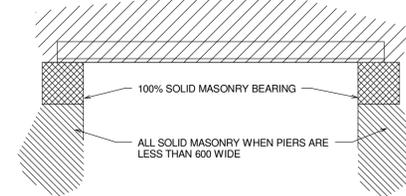
LINTELS (NON LOAD BEARING BLOCK WALL)

OVER ALL OPENINGS IN MASONRY WALLS PROVIDE THE FOLLOWING LINTELS, UNLESS OTHERWISE SHOWN.

BLOCK WYTHES

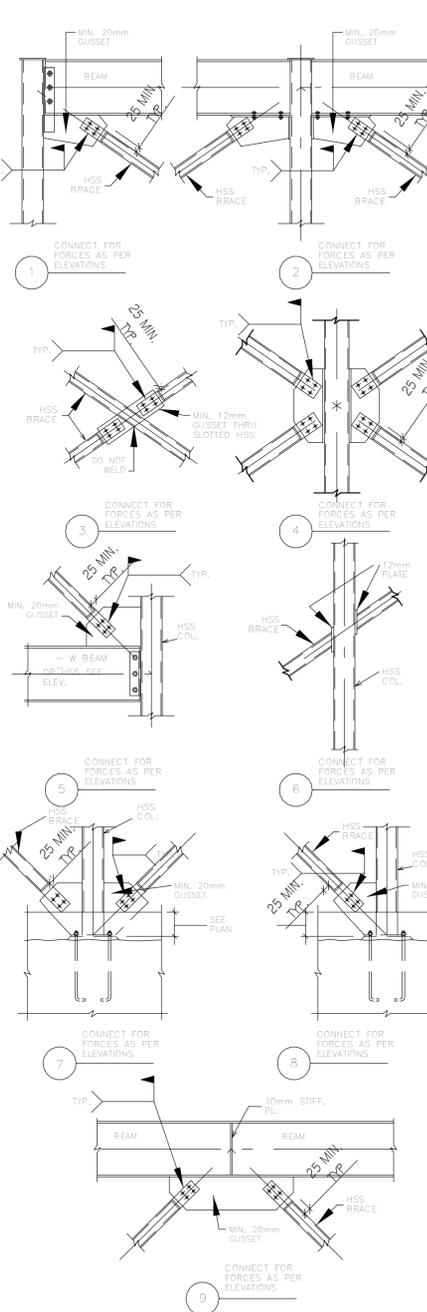
	140 (6") WALL	190 (8") WALL	240 (10") WALL	290 (12") WALL
CLEAR SPAN mm (ft-in)				
UP TO 1200 (4'-0")	2 - L64x64x6.4 LLV	2 - L89x89x7.9 LLV	L89x89x7.9 + L127x89 x 7.9 LLV	3 - L89x89x7.9 LLV
1201 TO 1800 (4'-0" TO 6'-0")	2 - L89x84x6.4 LLV	2 - L127x89x7.9 LLV	L127x89x7.9 LLV + L127x127x7.9 LLV	3 - L127x89x7.9 LLV
1801 TO 2400 (6'-0" TO 8'-0")	2 - L89x64x7.9 LLV	2 - L127x89x9.5 LLV	L127x89x9.5 LLV + L127x127x9.5 LLV	3 - L127x89x9.5 LLV
2401 TO 3000 (8'-0" TO 10'-0")	2 - L89x64x9.5 LLV	2 - L152x89x9.5 LLV	L152x89x9.5 LLV + L127x127x9.5 LLV	3 - L152x89x9.5 LLV

- WELD BACK TO BACK ANGLES TOGETHER TOP AND BOTTOM WITH 5mm (3/16") FILLET 50mm (2") LONG AT 450mm (18") MAXIMUM CENTERS.
- MINIMUM BEARING FOR STEEL LINTELS SHALL BE 150mm (6") AND BLOCK LINTELS SHALL BE 200mm (8").
- FOR WALLS OVER 300mm (12") THICK ADD ONE ANGLE FOR EACH ADDITIONAL 100mm (4") OF WALL THICKNESS OR PORTION THEREOF.
- FOR LINTELS ABUTTING STEEL COLUMNS, CONC WALL OR COLUMNS PROVIDE L 90x80x10 SHELF ANGLE.
- FILL VOIDS OF LINTEL BLOCK WITH 12.5 MPa GROUT MIN.
- USE MASONRY LINTELS IN ALL FIRE RATED MASONRY WALLS - SEE ARCH DWG. FOR WALL RATINGS.
- FOR 140 BLOCK USE BLOCK LINTELS.
- ALL STEEL LINTELS AND SHELF ANGLES IN THE EXTERIOR MASONRY SHALL BE HOT DIP GALVANIZED.



LINTEL BEARING ON MASONRY

TYPICAL BRACE CONNECTION DETAILS



Consultant:

ARCHITECT:

SAPLUS ARCHITECTS INC.
60 ST. CLAIR AVE E., SUITE 806
TORONTO, ONTARIO M4T 1N5
P: 905.510.0595



No.	Description	Date
2	ISSUED FOR BUILDING PERMIT	JUNE 17, 24
1	ISSUED FOR 50% PROGRESS	MAY 30, 24
Issues / Revision		

CONFIRM ALL GRIDS, DIMENSIONS AND ELEVATIONS WITH ARCHITECTURAL DRAWINGS. DO NOT SCALE DRAWINGS.

CONTACT ARCHITECT AND ENGINEER FOR ANY DISCREPANCIES.

ONLY LATEST APPROVED DRAWINGS TO BE USED FOR CONSTRUCTION.

ALL DRAWINGS REMAIN THE PROPERTY OF THE CONSULTANT.

AMR

AMR ENGINEERING LTD.
STRUCTURAL ENGINEERS
920 Alness Street, Suite 205,
Toronto, ON M3J 2H7
(416) 551 - 1611

Project North

Stamp:



Project :

WAREHOUSE AND OFFICE HEADQUARTERS

45 BLOWERS CRES
AJAX, ON L1Z 0N4

Drawing Title:

GENERAL NOTES & TYPICAL DETAILS

Date: 30-05-2024

Drawing No.

Scale: As Noted

S502

Drawn By: Author

Checked By: Checker

Job No: 2280